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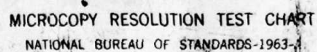
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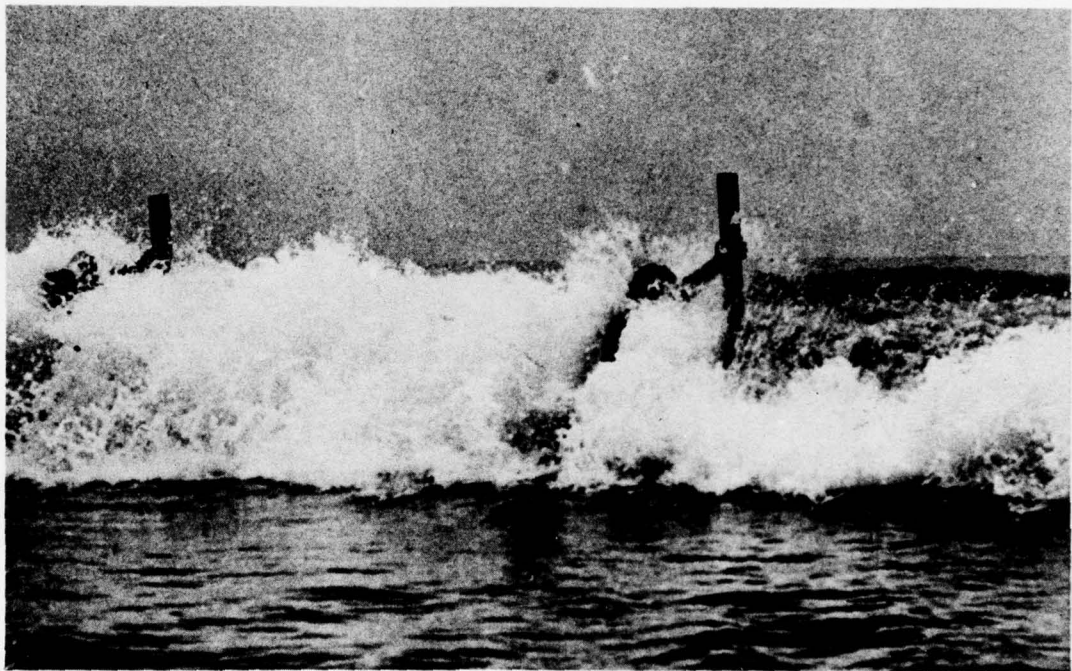


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Technical Report No. 18-CRD

SUSPENDED SEDIMENT IN BREAKING WAVES

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20. ABSTRACT CONTINUED

→ but on breaker type, which can be quantified to reasonable certainty by the simple ratio, d_b/H_b , relative wave height.

$$d_{\text{sub } b} / H_{\text{sub } b}$$

SUSPENDED SEDIMENT

IN BREAKING WAVES

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Department of Geology
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April 1979

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TABLE OF CONTENTS

	<u>Page no.</u>
Abstract	1
Acknowledgements	3
Introduction	4
Methods for Measuring Suspended Sediment	6
Direct vs. Indirect Measurement	6
Sampling Problems in the Surf Zone	9
The Simultaneous Water Sampler	9
Previous Results	12
Laboratory and Theoretical Studies	12
Field Measurements	16
Study Area	21
Experimental Design	26
Introduction	26
Suspended Sediment Measurements	28
Beach Surveying, Sample Positioning and Identification	34
Laboratory Procedures	35
Suspended sediment samples	35
Size analysis of suspended sediment	38
Computer Coding the Data	39
Rejection of Data	40
Results and Discussion of the Data	43
Introduction	43
Overall Means	43
Tests of Concentration vs. Breaker Type Parameters	48
Introduction	48
Galvin's Parameter (B_b)	51
Battjes' Parameter (ξ)	51
Parameter BRKER	54
Relative wave height (d_b/H_b)	57
Sorting the data by d_b/H_b	57
Tests of Concentration vs. Wave Process Parameters	60
Breaker height (H_b)	61
Wave period (T)	63
Longshore current velocity	63
Distance from breakpoint	66
Other process variables	66
Size Distribution of Suspended Sediment	68
Size distribution with respect to sample elevation	68
The relationship of suspended sediment grain size to wave parameters	71
Statistical Testing	75
Correlation Analysis	75
Multiple Regression	78

Discussion	81
Limitation of the Data	81
Advantages of the Data	82
Controlling Factors of Sediment Suspension	84
Implication of Results	90
Equilibrium profiles	90
Applicability of solitary theory to surf problems	92
Importance of suspended sediment to total long- shore transport	92
Conclusions	94
References Cited	96
Appendix A - Photo Atlas of Waves Sampled and Suspended Sediment Data	100
Appendix B - 1. Listing of Computer Variables	134
2. Example SAS76 Program Listing	135
3. Data Printout for Selected Observations and Variables	136
Appendix C - 1. Grain Size Data	142
2. Representative Size Frequency Curves	144
Appendix D - Correlation Coefficients	153

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LIST OF ILLUSTRATIONS

<u>Figure</u>	<u>Page</u>
1. Sampling suspended sediment (SS) in a breaking wave	4
2. Examples of direct SS concentration measuring devices	7
3. Examples of indirect SS concentration measuring devices	7
4. Simultaneous water sampler	10
5. SS distribution from Kana (1976a; 1977)	13
6. SS distribution from Inman (1977)	13
7. Bubble concentration in breaking waves from Miller (1976)	15
8. SS vs. elevation and wave height from Fairchild (1977)	17
9. Turbidity changes measured by the almometer	19
10. Frequency of occurrence of sand bursts	19
11. Location map near Charleston, S. C.	22
12. Location map of stations near Price Inlet, S.C.	22
13. Bulls Island	23
14. Capers Island	23
15. Beach profiles - station BU2	24
16. Beach profiles - station CA1	24
17. Size frequency curves - bottom sediments	25
18. Sketch of sampling arrangement	27
19. Simultaneous water samplers being "rigged"	29
20. Simultaneous water sampler ready for use	29
21. Photo sequence of a typical wave sampled	31
22. Photo sequence of transferring water samples	32
23. Custom beach cart	33
24. Example field data form	36
25. Sketch-filtering apparatus	37
26. Photo-filtering apparatus	37
27. Normal probability distributions	41
28. Normal probability SS10 and SS30	42
29. Galvin's (1968) breaker classification	44
30. Breaker types common near Price Inlet	45
31. Suspended sediment distribution by elevation and breaker type	46
32. SS10 vs. B_b	52
33. SS10, SS30 and SS60 regression models on B_b	52
34. SS10 vs. XI	53
35. SS10, SS30 and SS60 regression models on XI	53
36. SS10, SS30 and SS60 vs. BRKER	56
37. SS10, SS30 and SS60 vs. beach slope	58
38. SS10, SS30 and SS60 vs. d_b/H_b	59
39. SS10 vs. wave height by wave type	62
40. SS10 vs. wave period by wave type	64
41. SS10 vs. longshore current velocity by wave type	65
42. SS10 vs. distance from the breakpoint	67
43. SS size by elevation from Fairchild (1977)	69
44. Example size frequency distribution of SS10, SS30 and SS60	70
45. Size frequency of SS10, SS30, SS60 and corresponding bed sample	72
46. Positively skewed SS10 samples	73
47. SS mean size by elevation, breaker type and wave height	74

<u>Figure</u>	<u>Page</u>
48. Distribution of velocity in breaking waves (Iverson, 1952) ...	86
49. General linear models of SS	88
50. Dean's (1973) model of onshore/offshore transport	90
51. Effect of breaker type on equilibrium profile	93

LIST OF TABLES

<u>Table</u>	<u>Page</u>
1. Distribution of SS samples by wave type and height	36
2. Number of samples coded and retained	42
3. Mean values of selected measured variables	47
4. Size data from Fairchild (1977)	69
5. SS grain size statistics by station and date	70
6. SS grain size statistics by breaker type	74
7. Correlation coefficients - SS and independent variables	76
8. Correlation coefficients - SS size and independent variables ...	76
9. Improved correlation coefficients using means	78
10. Multiple regression models	80

ABSTRACT

Suspended sediment concentration was measured in 235 breaking waves on undeveloped beaches near Price Inlet, South Carolina, U.S.A., using portable in situ bulk water samplers. The purpose of the study was to determine what factors control the distribution of suspended sediment in the breaker zone. As many as 10 instantaneous 2-liter water volumes were obtained in each wave for a total of 1500 samples. Concentrations of suspended sediment were determined at fixed intervals of 10, 30, 60 and 100 cm above the bed for various surf zone positions relative to the breakpoint. The majority of waves sampled during 22 days in June and July, 1977 were relatively long crested, smooth, spilling to plunging in form, with breaker heights ranging from 20 to 150 cm. The beaches sampled are gently sloping (mean beach slope = 0.015), fine-grained (mean grain size = 0.18 mm) and densely compacted with an absence of small scale bed forms.

Suspended sediment in the breaker zone is composed of two fractions;... a continuous wash load mode above 60 cm from the bed and an intermittent mode of coarse bed material entrained to lower levels during certain wave conditions. Mean concentration decreases exponentially above the bed to approximately the 60 cm elevation, then maintains a generally constant level up to the water surface. Suspended sediment concentration at the study sites ranged over 3 orders of magnitude up to approximately 10 grams per liter.

The principal factor controlling suspended sediment concentration at a point in the breaker zone is breaker type. Plunging waves typically entrain one order more sediment than spilling breakers. Breaker type for these data can be reasonably quantified as a continuous variable on the basis of relative wave height, d_b/H_b . Plunging waves near Price Inlet occur at $d_b/H_b < 0.89$; whereas spilling waves generally break at $d_b/H_b > 1.10$. Mean concentration increases with decreasing d_b/H_b according to

$$\text{Log}_{10}(\text{SS}_{10}) = 17.4 - 1.7 d_b/H_b,$$

where SS_{10} is suspended sediment concentration at 10 cm above the bed. This theorized model accounts for almost 60% of the variation in mean concentration by d_b/H_b .

Secondary controlling factors of concentration also include distance relative to the wave breakpoint, beach slope and wave height. Mean suspended sediment in the breaker zone reaches a maximum several meters landward of the breaker line peaking more sharply in plunging than in spilling waves. For the range of slopes in the present study (.004-.040), mean concentration increases according to the model:

$$\text{Log}_{10}(\text{SS}_{10}) = .22 + 14.5 m,$$

where m is dimensionless beach slope.

The relation between wave height and concentration depends on breaker type. There is little or no dependency of concentration on wave height for spilling waves....However, for plunging waves, suspended sediment concentration at a point decreases with increasing wave height.

For the present data collected under moderate swell conditions, suspended sediment concentration is independent of wave period, long-shore current velocity, wind velocity and any breaker type parameter involving wave steepness (H_b/L_o).

Although the amount of variation in mean concentration accounted for only ranges up to 65%, these data support the notion that sediment suspension in the surf zone is statistically predictable. The importance of breaker type on concentration suggests that transport of sand in the surf zone is less dependent on wave height and wave steepness than on relative wave height, d_b/H_b .

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INTRODUCTION

In June and July, 1977, a study was conducted at two beach sites near Price Inlet, South Carolina to determine the amount of suspended sediment occurring in the breaker zone under moderate swell conditions. The purpose of this experiment was to obtain fundamental information on sand suspension in breaking waves in order to construct a predictive model of sediment concentration. While these data provide the nucleus for a first order model, they represent rather limited conditions with regard to wave type, wave height, beach slope, and sediment grain size and bed compaction.

The principal hypothesis that was tested can be summed up as follows:

Is the entrainment of sand in breaking waves a random process as variable as the turbulence field in the surf zone -- or is it predictable to some degree on the basis of certain easily measured wave parameters?

To test this, an unique portable water sampling device was designed, and field techniques were developed for obtaining suspended sediment samples directly in breaking waves up to 150 cm high (Figure 1).

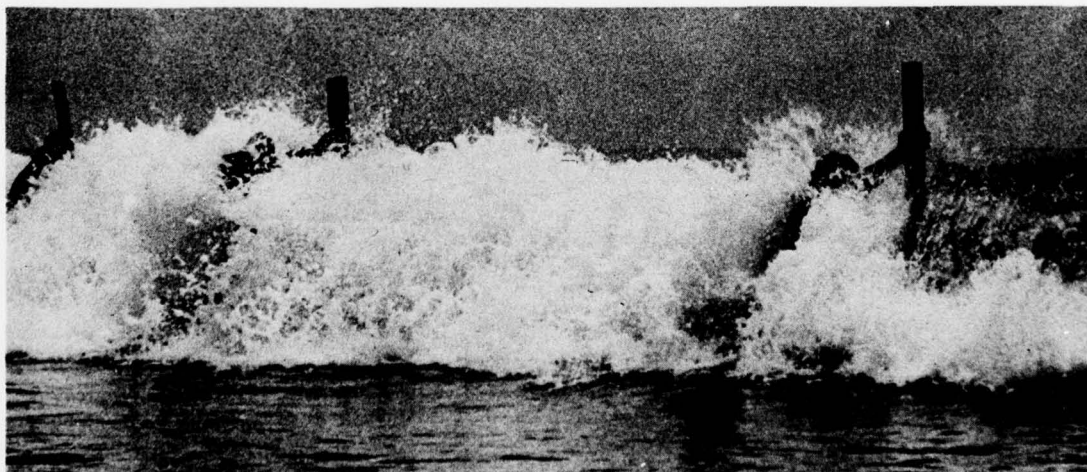


Figure 1. Obtaining samples of suspended sediment in a 110 cm high plunging breaker using portable in situ bulk water samplers (see Fig. 4).

The data base for the present experiment includes over 1500 suspended sediment samples collected in 235 individual breakers covering a normal range of swell conditions typical to the South Carolina coast. In addition, numerous wave process and beach profile observations were obtained. Field techniques were basically the same as those applied by Kana (1976a) in an earlier study, but several changes were made to improve control over positioning of the samplers, measurement of wave parameters and photography of the sampling. The experimental design entailed selecting a well defined breaking wave, positioning all suspended sediment samples with respect to the breakpoint and the bed, photographing the wave as it progressed toward shore and measuring the corresponding wave process parameters. The data was evaluated by statistically testing individual and mean values of suspended sediment concentration (dependent variable) against process and location parameters (independent variables).

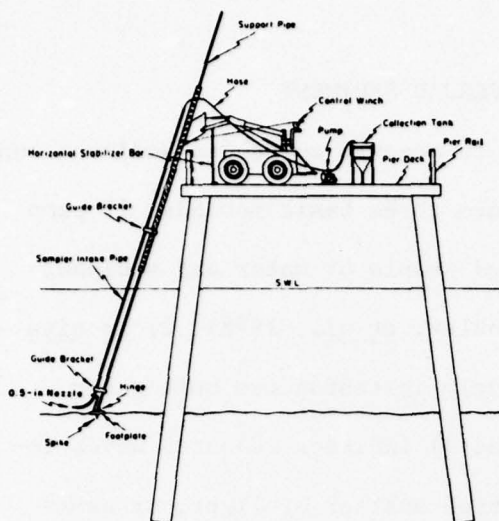
METHODS FOR MEASURING SUSPENDED SEDIMENT

Several techniques have been used to measure suspended sediment concentration in breaking waves. There are three basic methods: 1) pump systems for obtaining a time-integrated sample of water and sediment (Watts, 1953; Fairchild, 1972; and Coakley, et al., 1978); 2) in situ collecting traps for obtaining relatively instantaneous bulk water samples (Kana, 1976b; Inman, 1977); and 3) indirect measures which relate turbidity to light attenuation, back scatter of light, or gamma absorption (Hom-ma, et al., 1965; Hattori, 1969; Horikawa and Watanabe, 1970; Kennedy and Locher, 1972; Basinski and Lewandowski, 1974; and Brenninkmeyer, 1976a). Examples of several systems are shown in Figures 2 and 3. Of the indirect monitors of turbidity, only Brenninkmeyer's has been designed for use in the surf zone. There are certain disadvantages to any of these techniques, most important of which is the influence of the sampling apparatus on the flow field. According to Inman (1977), any device which remains fixed to the bed, or utilizes a supporting structure or pier, is likely to monitor artificially-induced suspensions.

Direct vs. Indirect Measurement

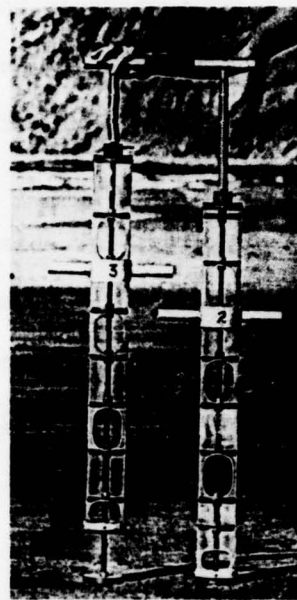
Pump samplers and in situ bulk water samplers have the advantage of providing a direct sample of suspended sediment which can be used for multiple laboratory analyses. However, they only provide a sample for one instant, or a short period, in time, making it difficult to construct a time series of turbidity changes.

The pump samplers used by Watts (1953) and Fairchild (1972) required a pier for support and provided one time-averaged water sample per run. In general, pumping times ranged from 45 seconds to 3

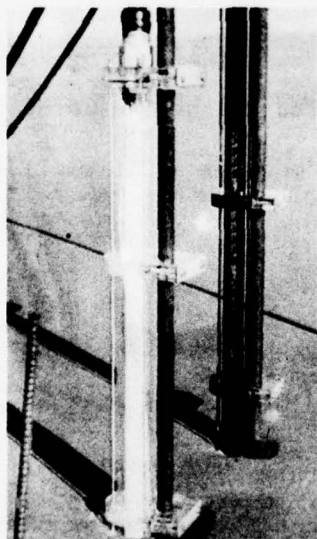


2a. Tractor mounted, pump sampler on a pier (From Fairchild, 1977; Fig. 5).

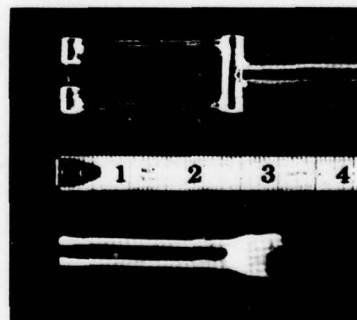
Figure 2. Examples of direct suspended sediment concentration sampling devices used in studies in waves.



2b. Portable, in situ bulk water sampler which "cores" the water column to provide multiple serial samples (photo courtesy of D.L. Inman, Scripps).



3a. Almometer, an array of photo-electric detectors to measure turbidity (From Brenninkmeyer, 1976b; Fig. 1)



3b. The Iowa Sediment Concentration Measuring System (ISCMS). Electro-optical probes used to detect temporal changes in suspended sediment in the laboratory (From Locher, et al., 1977; Fig. 2).

Figure 3. Examples of indirect suspended sediment concentration measuring devices used in studies in waves.

minutes making it possible to measure a mean concentration at a single point. More recently, Coakley, et al., (1978) have reported development of a computerized "robot" sled system for pumping and retaining up to 30 time-integrated samples at pre-programmed positions in the surf zone. These systems have the distinct advantage of remote operation, but do not isolate the effect of a single wave or detect rapid short period bursts of sand from the bed.

Indirect monitors of concentration, such as the almometer (Brenninkmeyer, 1976a), have the advantage of providing detailed time series information on turbidity changes, but most of these devices are difficult to calibrate for field use. They are generally of two types: photoelectric (e.g. Homa-ma, et al., 1965; Brenninkmeyer, 1976a), in which turbidity fluctuations cause voltage changes which are detected by photo cells and related to a calibration curve to determine concentrations; or electro-optical (e.g. Kennedy and Locher, 1972) in which actual particle counts between two closely spaced probes are measured and related to the flow field to determine concentration. Calibration of these devices is done in the lab, but in field use, such external conditions as cloud cover, air entrainment in breaking waves, and the presence of varying amounts of organic matter in the water column, affect turbidity and, therefore, the output of these devices. Also, in some cases, the threshold for detecting sediment bursts from the bed is significantly higher than the typical concentrations found in the surf zone. For example, the concentrations reported by Watts (1953) for a California beach and Kana (1976a) for South Carolina are typically less than 5 grams per liter (g/l), whereas the threshold for detection of suspended sediment by the almometer is calibrated at 5-10 g/l (Brenninkmeyer, 1976a).

Sampling Problems in the Surf Zone

The main considerations for sampling suspensions in the surf zone are considered to be the following:

- 1) The concentration will depend on a combination of continuous suspension wash load of fine grained particles and an intermittent suspension of coarse bed material. Of these two components, the intermittent suspension is of greater interest and uncertainty.

- 2) Samples must be collected with respect to elevation above the bed where the intermittent suspension mode originates.

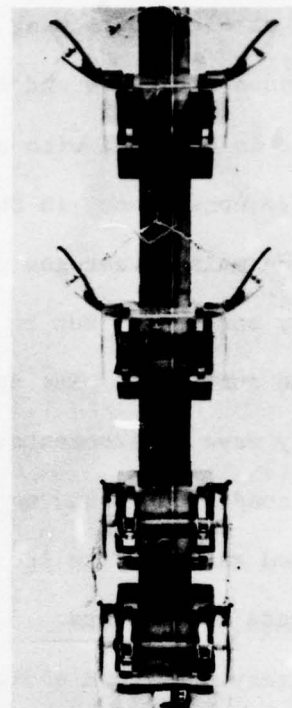
- 3) Suspended sediment will fluctuate rapidly with the wave period, and bursts of sand from the bed will generally be of short duration.

With these criteria in mind, a portable sampler was designed to collect multiple, relatively instantaneous bulk water samples for determining suspended sediment and isolating the effect of an individual wave on sand entrainment. It was designed to operate simply and at a reasonable cost.

The Simultaneous Water Sampler

The apparatus used in the present study collects several closely spaced simultaneous water samples in a vertical array above the bed (Kana, 1976b). It consists of a 2 meter-long mounting pole, support brackets and several 2-liter cast acrylic bottles closed off by hinged doors (Fig. 4). A spring loaded trigger assembly which holds each bottle door is mounted to the support pole. At the base of the trigger is a footpad which can be pushed up to open the trigger and simultaneously release all bottle doors.

Figure 4. Apparatus used to collect water samples in the surf zone. A 2 m-long pole supporting several 2 liter bottles is emplaced vertically in the surf zone. When thrust into the bed, a footpad moves the rigger assembly up, simultaneously tripping each bottle. Top two bottles are rigged for sampling. Bottom bottles are in the tripped position



To ready the sampler for use, the operator opens the bottle doors and attaches them to a trigger assembly on the mounting pole, similar to rigging a Van Dorn-type water sampler. Then as the bottles are held open, the device is carried into the surf zone and positioned vertically above the bed until the sampling instant. At the desired sampling time, the apparatus is thrust into the bed, forcing the trigger open and allowing the bottles to shut simultaneously, trapping each sample.

The device has a relatively fast response time of less than one-half second, remaining off the bed until the sampling instant. Tests have shown that the collecting bottles are drawn shut before sediment thrown up by the apparatus reaches each sampling position. The typical array of samples collected in this study were centered at 10, 30, 60 and 100 cm above the bed. Because of the relatively broad, stubby

shape of each collecting bottle, the lowermost sample obtains sediment suspended between 4 and 16 cm above the bottom. Positioning of each sample is constant with respect to the bed, making it possible to achieve consistency in the results.

The main advantages of this device are its portability, efficiency and cost. But it has the disadvantage of requiring an operator in the surf zone. The sampler is obviously inappropriate for high energy wave environments; however, it has been used for almost all wave conditions found on the South Carolina coast. Wave heights sampled ranged up to 160 cm, including waves generated during a moderate local storm.

Every suspended sediment sampler or meter used, to date, has its limitations. According to Inman (1977), the best way of overcoming present sampling problems is to deploy turbidity meters in combination with portable in situ bulk samplers activated by swimmers. This would provide a physical sample to check the calibration of the meters. The task of coordinating the two measurement techniques, however, is formidable and requires more control and expense than is presently warranted. Our understanding of the causes of suspended sediment variation is limited and would be best served by additional detailed field studies at sites differing in wave climate and beach morphology from West Coast or East Coast beaches. Direct samples of suspended sediment are needed to insure reliability of the data and allow quantification of the effect of wave processes on concentration. The techniques applied in this experiment offer a less than ideal, but efficient and inexpensive way of study suspended sediment in moderate wave conditions.

PREVIOUS RESULTS

It is generally recognized from laboratory studies, that in oscillatory flow, suspended sediment concentration decreases exponentially above the bed (Hattori, 1971; Kennedy and Locher, 1972; and MacDonald, 1977). Field measurements in the surf zone by Kana (1976a) and Inman (1977) (Figs. 5 and 6) tend to confirm this relation. A relatively constant suspension wash load of fine-grained particles exists throughout the water column in the nearshore. However, in the breaker and swash zones, intermittent suspensions of relatively coarse bed material are thrown up by waves to cause the observed vertical distribution of concentration. The frequency and magnitude of these intermittent suspensions are of primary interest because of their importance in the transport of sand on beaches. In general, the timing of bursts of sediment from the bed corresponds to the time of wave breaking, with some delay as the particles lag behind the water motion (Hattori, 1969; Brenninkmeyer, 1976b).

Laboratory and Theoretical Studies

Most of our knowledge of the mechanics of sediment suspension under oscillatory flow and breaking waves is semi-quantitative, based on laboratory experiments performed under controlled conditions. Significant research has been conducted at the University of Iowa, Institute of Hydraulic Research (e.g. Kennedy and Locher, 1972; Locher et al., 1976; and Nakato, et al., 1977), University of California, Berkeley, Hydraulic Engineering Laboratory (e.g. Das, 1971 and MacDonald, 1977) and the University of Tokyo, Department of Civil Engineering (e.g. Homma, et al., 1965; Hattori, 1969; and Horikawa and Watanabe, 1970). For the most part, these experiments deal with sediment suspension under non-breaking progressive or standing waves.

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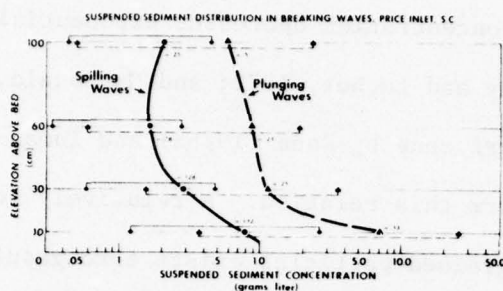


Figure 5. Mean concentration by elevation above bed for approx. 450 suspended sediment samples obtained using the *in situ* bulk water sampler in Fig. 4 (From Kana, 1976a).

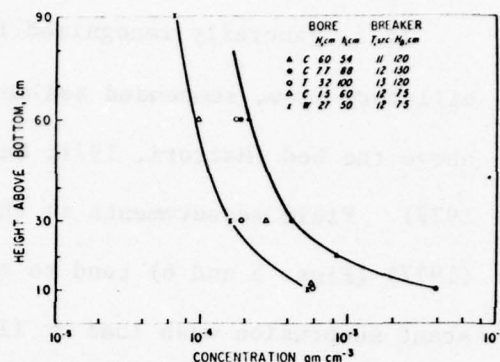


Figure 6. Distribution of suspended sediment at 10, 60 and 90 cm above the bed measured using the *in situ* bulk water sampler shown in Fig. 13c (From Inman, 1977).

The results from the Iowa group are illustrative of some of the problems involved in measuring sediment suspensions in oscillatory flow. Using small electro-optical probes for measuring turbidity, hot-wire anemometers for obtaining current velocity profiles and a computer data control system for signal averaging to obtain mean, periodic and random concentration and velocity components, Nakato, *et al.*, (1977) were able to identify the suspended sediment distribution over rippled beds under non-breaking waves. They found that suspensions were proportionately higher over the crest of ripples than over the troughs. They also reported that a signal averaged concentration at any position near the bed has four prominent peaks during the wave cycle, originating from the movement past a fixed point of sediment held in suspension by the eddy produced in the lee of each ripple during each half cycle of wave motion. Yet, despite the sophistication of their experiments, they admit to having difficulty in determining the reference concentration needed by engineers to formulate

predictive models of sediment transport.

MacDonald (1977), using an oscillating flume, was the most recent investigator to find that mean sediment concentration decreases exponentially above the bed, more or less in agreement with Hattori (1971). Unfortunately, there has been no emphasis on laboratory studies of sediment entrainment in breaking waves.

Perhaps the most helpful laboratory experiment for the present study is one by Miller (1976), in which air entrainment, not suspended sediment concentration, was measured in breaking and non-breaking oscillatory waves. Miller (1976) photographed various types of breaking waves in the wave tank in order to contour the distribution of air bubble concentration. An example of his results is shown in Figure 7. Note that plunging waves, which tend to break more violently, entrain air all the way to the bed; whereas spilling waves break gradually, confining the air entrainment to the surface. This indicates that the turbulence field is significantly different between these two basic wave types, and, by inference, their capacity to entrain sediment from the bed should differ markedly. In agreement with the above, Fuhrboter (1970) also concluded that plunging breakers dissipate their energy over a narrower portion of the surf zone than spilling breakers, as indicated by the distribution of air bubbles.

The laboratory studies mentioned above bring up some important points with respect to the design of the present study. First, it is likely that an exponential decay of concentration above the bottom will occur in breakers, making it imperative that positioning relative to the bed is consistent from one sample to another. A change

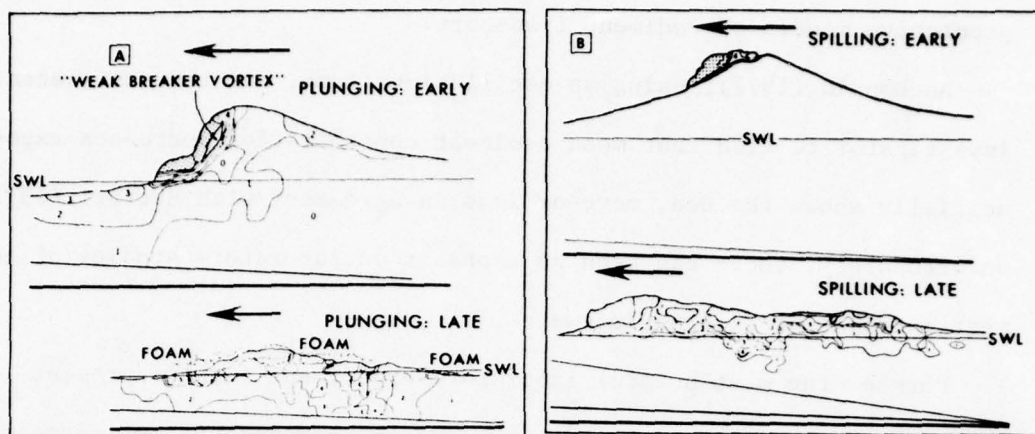


Figure 7. Cross-sectional contour diagrams of bubble concentration in plunging (A) and spilling (B) breakers. Numbers represent percentage estimate of the number of 1 mm scale units containing one or more bubbles (on a scale of 1 to 10). A 5 means 50% of the scale units contain bubbles, a 10 represents 100% bubble concentration. Note that bubbles penetrate to the bed in plunging waves, but remain close to the surface in spillers. This suggests more turbulence reaches the bed in plunging waves, contributing to the observed higher concentrations of suspended sediment for this breaker type (Miller, 1976).

in elevation of a few centimeters may cause a significant change in the concentration. Secondly, the amount of sediment entrained will depend on the turbulence field. This will be affected not only by the total wave energy available, but by the type of breaker. Finally, if the suspensions in the field are periodic, sampling consistency with respect to passage of the wave will be required.

Field Measurements

Very few workers have directly measured suspended sediment concentration in the surf zone. Watts (1953) and Fairchild (1972, 1977) used pump samplers from ocean piers; while, more recently, Coakley et al. (1978) used a bottom sled with preprogrammed submersible pump and collecting system. Each of these techniques provides time-averaged water samples from which sediment is extracted to determine the mean concentration of suspended sediment during the sampling interval. Watts and Fairchild generally collected a 40 gallon sample by pumping for approximately 3 minutes; whereas Coakley's apparatus pumps 2 liter samples in approximately 45 seconds.

Of these studies, the most detailed results are reported in Fairchild (1977), in an updated version of an earlier paper (Fairchild, 1972). Working from ocean piers at Ventnor, New Jersey and Nags Head, North Carolina, he collected over 700 time-averaged water samples in 1964 and 1965 seaward and landward of the breaker zone, using the tractor mounted pump sampler shown in Figure 3. Fairchild, sampling in waves from 40 to 120 cm high, obtained concentration values ranging over 3 orders of magnitude to a maximum of 4.0 parts per thousand. The sampler intake was varied between 8 cm and 75 cm above the bed.

Despite a great amount of scatter in the data (Fig. 8), Fairchild isolated several trends, including:

1. Suspended sediment increases slightly with breaker height;
2. Concentration decreases away from the breakpoint in both the seaward and landward direction;
3. Concentration decreases with elevation above the bed; and
4. Suspended sediment is more variable in low waves than high.

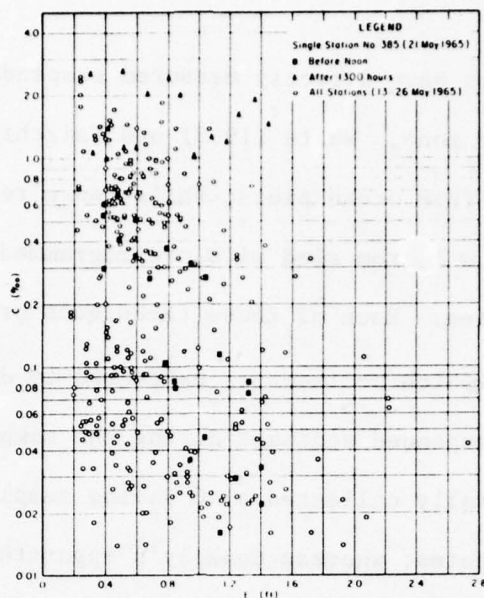


Figure 8a. Scatter plot of suspended sediment concentration (C ‰) vs. elevation above the bed (E - ft) for pump samples collected from an ocean pier at Ventnor, N.J. on 21 May 1965 (From Fairchild, 1977; Fig. 18).

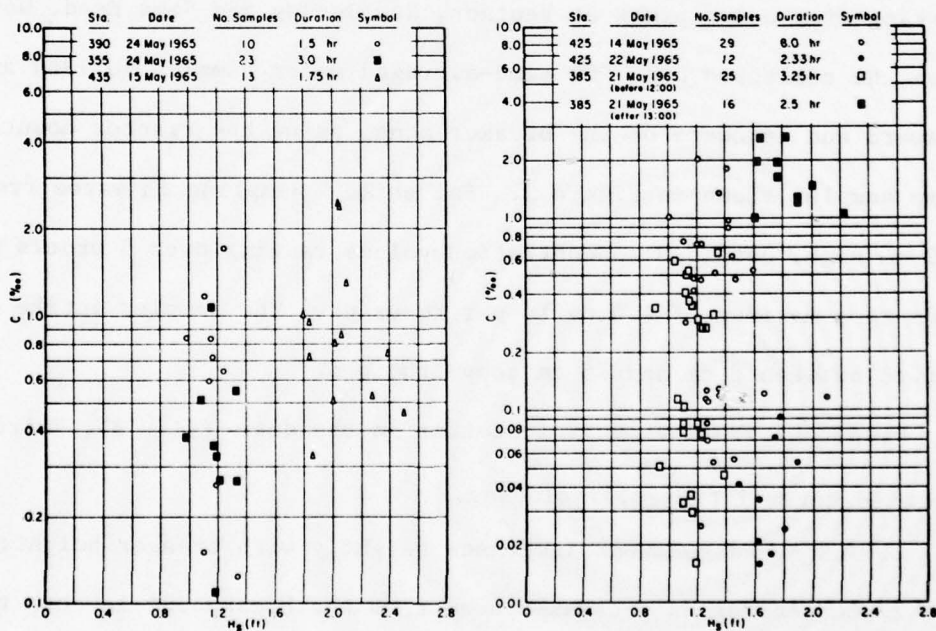


Figure 8b. General lack of relation between suspended sediment (C ‰) and significant wave height (H_s) for samples collected at Ventnor, N.J. (From Fairchild, 1977; Fig. 23).

Brenninkmeyer (1973) developed the almometer at the University of Southern California and has used it at Pt. Mugu, California and the Massachusetts and Georgia coasts to obtain detailed time series of turbidity fluctuations in the surf zone. The principle conclusions of his studies indicate that:

- 1) The zone of maximum suspension occurs near the still water level (the elevation the sea would maintain in the absence of wave action);

- 2) Suspensions of sand more than 15 cm above the bed are rare in the outer surf zone and appear to be influenced by the stage of the tide and the elevation of the ground water table;

- 3) Bursts of sediment from the bed or "sand fountains" are infrequent and have durations of the order 2 to 10 seconds, apparently as a function of the phase of incident waves; and

- 4) The elevation of the bed may change by up to 6 cm during the passage of a single breaker (Brenninkmeyer, 1974, 1976b).

Figure 9 shows a characteristic time series of concentration levels versus elevation above the bed from Brenninkmeyer's (1976b) study. At Point Mugu, California, Brenninkmeyer observed two principal frequencies of occurrence of sand bursts on the upper beach face. As shown in Figure 10, one is at 15 s apparently corresponding to the incident wave period, and another is at 21 s, possibly occurring due to constructive interference of shorter period wave trains.

Loenard and Brenninkmeyer (1978) have used the almometer during storm conditions at Nauset Beach, Massachusetts and, not unexpectedly, concluded that: 1) during storms, sediment suspensions are more prevalent; and 2) the frequency of movement decreases with elevation above the bed and in a seaward direction from the beach. They also

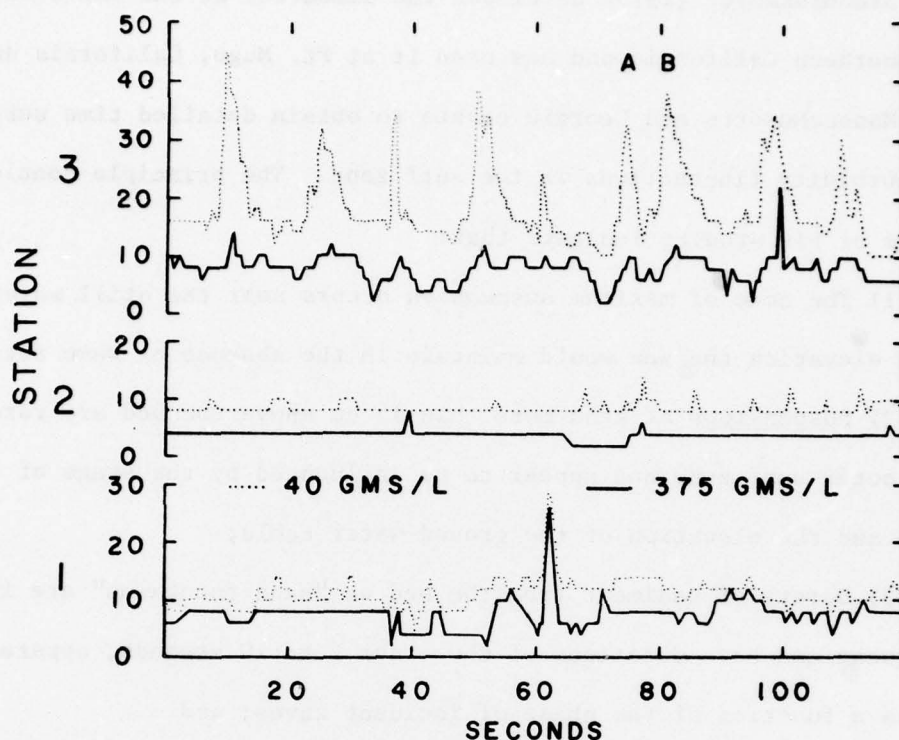


Figure 9. Example two minute record of turbidity fluctuations by elevation obtained from the almometer shown in Figure 3b. The solid line represents a constant value of 375 gm/l, and the dotted line is 40 gm/l. Record #1 (lower) is in the breaker zone; #2 is in the mid-surf zone; and #3 is in the swash zone near the still water level. Note: 1) Suspended sediment concentrations are highest in the swash zone; 2) bursts of high concentration are less frequent in the breaker zone; and 3) the gradient in concentration is generally steepest in the breaker zone (From Brenninkmeyer, 1976b; Fig. 14).

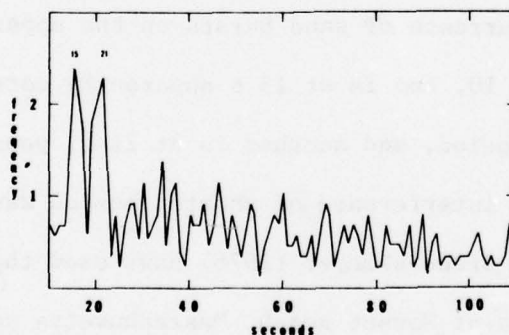


Figure 10. Frequency of occurrence spectrum of sand bursts on the upper beachface at Pt. Mugu, Calif., using the almometer. Dominant frequency of 15 s corresponds approximately to the incident wave period. The second mode may be due to constructive interference of shorter period waves (From Brenninkmeyer, 1976b; Fig. 7).

observed concentration inversions, which occur due to the shearing of tabular clouds of sediment moving in the upper layers. This produces a reverse gradient of higher concentration overlying a zone of lower concentration.

The almometer provides the best information to date on the time series of suspended sediment under field conditions. It suffers from imprecision of calibration and a high threshold of detection, but is an excellent device for obtaining remote information on the semi-quantitative aspects of sediment suspension.

The field measurements of Kana (1976a; 1977) and Inman (1977) were obtained from in situ bulk water samplers (Figs. 5 and 6). Kana's data includes approximately 700 concentration determinations obtained from serial arrays of water samples. The means plotted in Figure 5 represent the portion of data collected between 1 and 3 m landward of the breaker line (449 samples) and are divided according to breaker type (based on a visual wave classification). Kana's (1976a) data indicate that plunging waves entrain almost one order more sediment than spilling breakers. Although the data reported by Inman (1977) are minimal being based on relatively few samples, they appear to verify the exponential decay in concentration above the bed.

Despite limitations in all the studies mentioned, these previous experiments provide a starting base for the present study.

STUDY AREA

The field measurements for the present study were obtained at two beach stations (BU2 and CA1), each approximately 2 km from Price Inlet, on Bulls Island and Capers Island, South Carolina (Figs. 11-14). This portion of the South Carolina coast is under the influence of dominant waves from the northeast, causing net longshore transport to the south (estimated rate is $1.2 - 1.5 \times 10^5 \text{ m}^3/\text{yr}$; Kana, 1977). Wave energy is moderate with breaker heights ranging from 20 to 160 cm under non-storm conditions, with a mean of 60 cm.

The beaches at these two sites are composed of well-sorted, fine sand (mean diameter = 0.22 mm) and are gently sloping and relatively featureless (mean beach face slope = 0.018). During average swell conditions, the surf zone is approximately 50 meters wide, but due to the mean tide range of 1.5 meters, a much wider portion of the beach face is periodically exposed to the impact of breaking waves. The bed in the active surf zone is tightly compacted and rarely exhibits small scale bedforms. Slope changes along the beach face are minor and generally controlled by the formation of low amplitude ridges or bars parallel to shore. Figures 15 and 16 show representative beach profiles for each station measured by the Emery (1961) technique.

Sediment grain size along the profile can change significantly due to the presence of varying amounts of shell debris. For example, a series of size frequency curves for bottom sediments along profile BU2 (Fig. 17) show variations in sorting and mean grain sizes. Finest, best-sorted sediments are located at the middle of the berm (17a) and outer ridge (17f); coarser, more variable sediments containing shells are found at the step (17c) and along the runnel (17e).

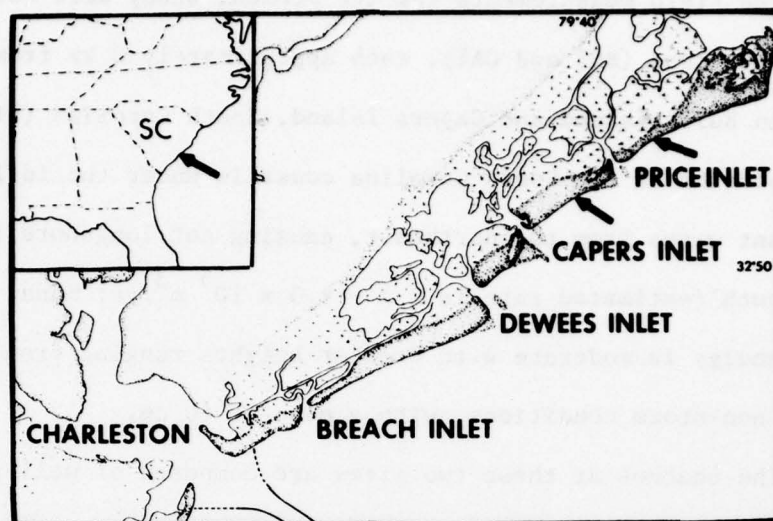


Figure 11. The study area located northeast of Charleston, South Carolina, U.S.A. Suspended sediment samples were collected at stations located along two undeveloped beaches near Price Inlet (arrows).

Figure 12. Detailed location map of primary sampling stations, BU2 on Bulls Island and CA1 on Capers Island. A secondary station, PI1, was used to collect some of the samples for size analysis. In general, BU2 has been stable and CA1 erosional over the past 4 yrs.

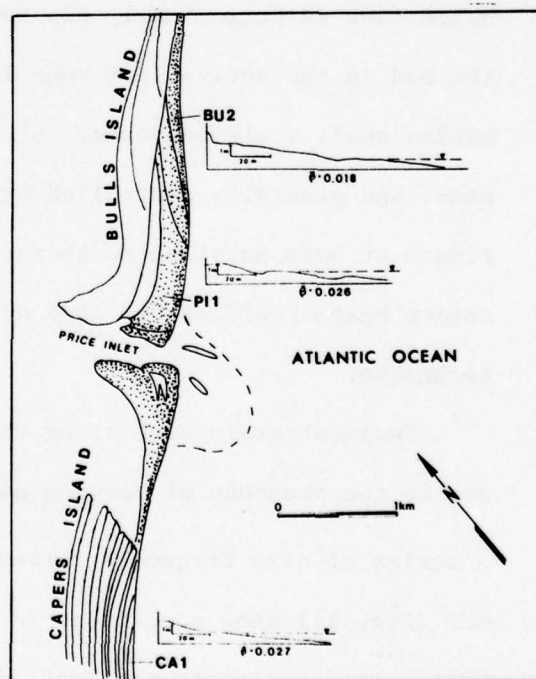




Figure 13. Oblique IR aerial photo of Bulls Island, looking northeast. Net sediment transport is southeast at $1.3 \times 10^5 \text{ m}^3/\text{yr}$. (Kana, 1977).



Figure 14. Oblique IR aerial photo of Capers Island looking southeast. Note erosion of antecedent beach ridges which has left a forest of driftwood along portions of the beach.

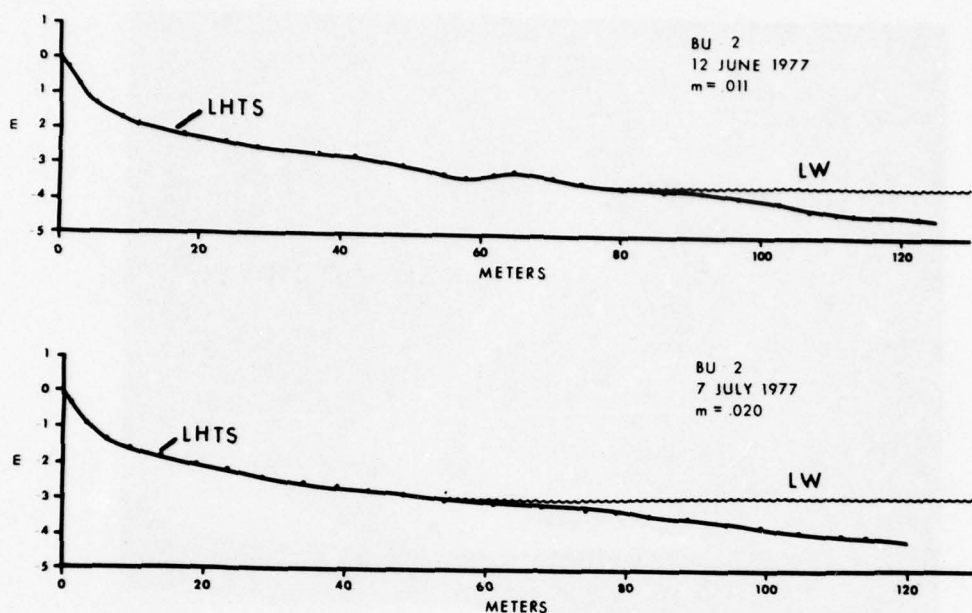


Figure 15. Beach profiles at station BU2 on 12 June (upper) and 7 July 1977 (lower) during the 1 month field experiment. The small amplitude ridge shown on the top profile was gone by 7 July.

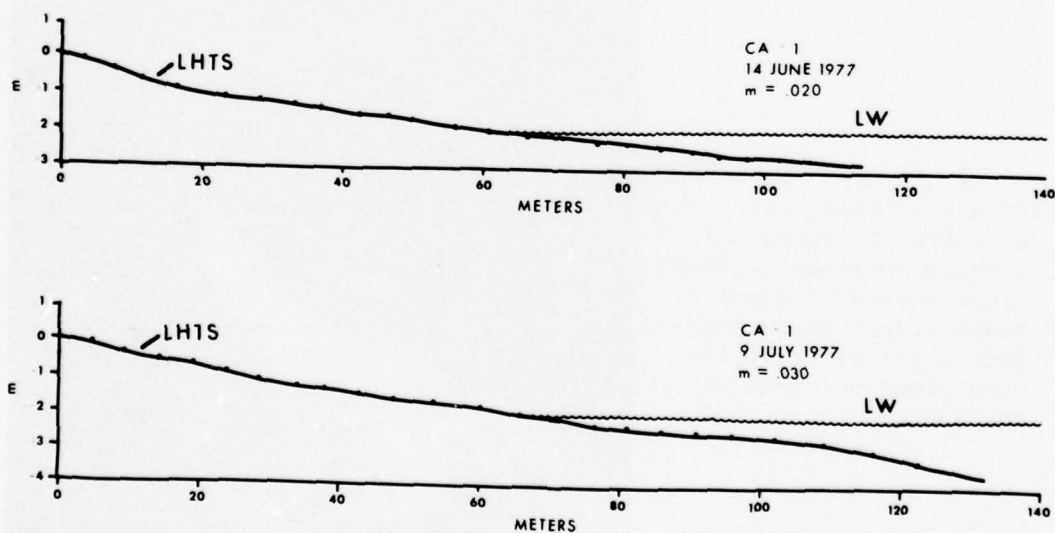


Figure 16. Profiles at station CA1 on 14 June and 9 July 1977, showing a generally featureless profile. Small amplitude ripples or bed forms are essentially absent at both stations.

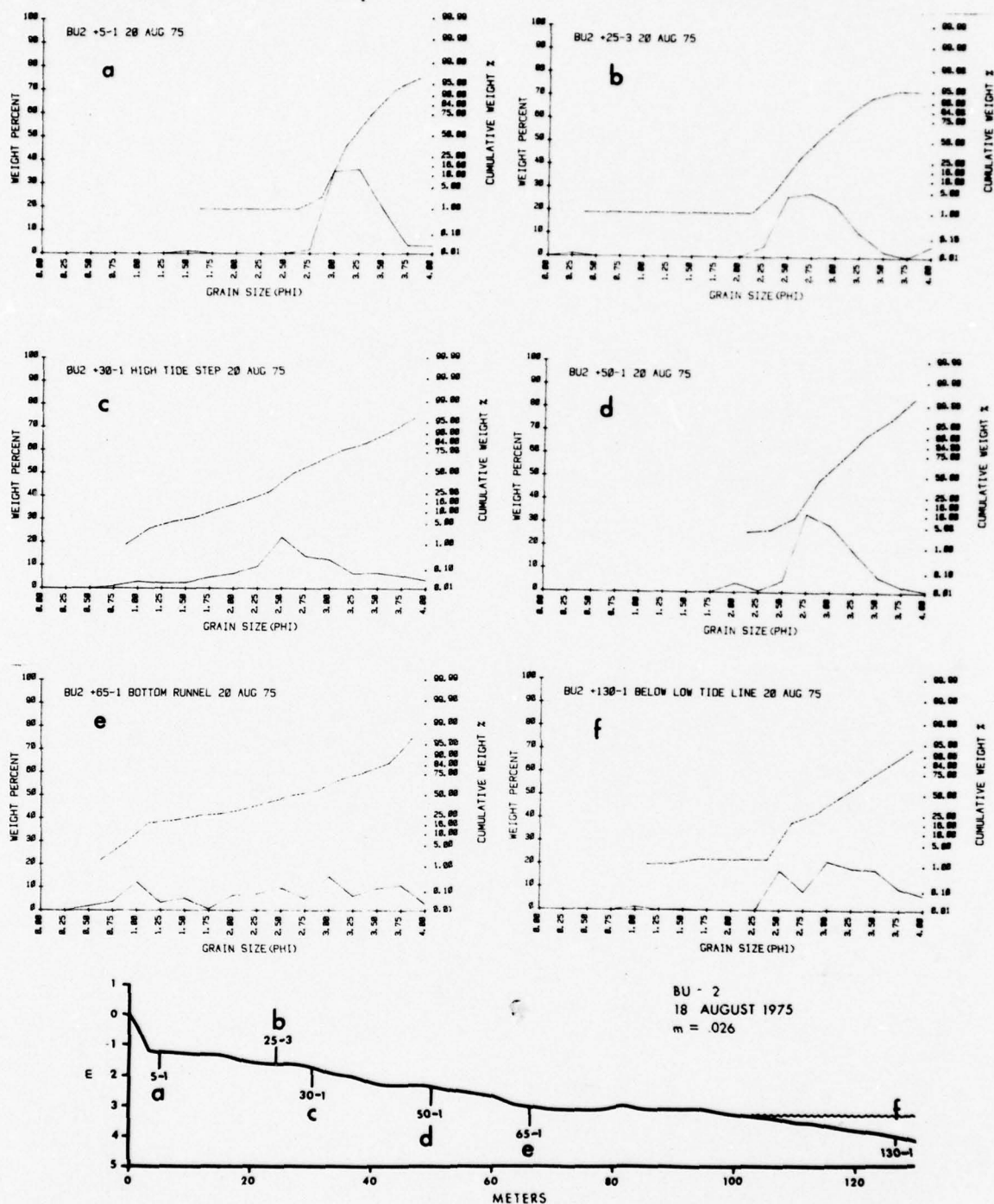


Figure 17a-g. Size frequency curves for bottom sediments along profile BU2 calculated from settling velocities. Sample locations are given in Fig. 17g in terms of a reference distance (e.g. 30-1 which corresponds to grain size distribution in Fig. 17c, labeled "BU2 +30-1 High Tide Step") where the distance is measured in meters from a benchmark on land. Note, finest, best-sorted sediments are located at the middle of the berm (17a) and outer ridge (17f); whereas coarser, more variable sediments containing shells are found at the step (17c) and along the runnel (17e).

EXPERIMENTAL DESIGN

Introduction

Most of the field data reported herein were collected during 22 sampling days in June and July 1977. Additional data collected in 1975 were used to expand the section on size analysis. Since the intent of the experiment was to isolate the effect of individual waves on sediment entrainment, an attempt was made to conduct the sampling under "ideal" swell conditions to facilitate identification and typing of the waves and reduce the variability due to external parameters such as local wind stress. Generally, at these beach sites, waves are more "classically" formed and long crested in the morning before the diurnal seabreeze is established. As onshore wind velocity increases, wave crests become shorter and more irregular, making it difficult to distinguish the breakpoint.

The beaches at these two sites were chosen for the experiment because their profiles are generally featureless, small scale bed-forms are essentially nonexistent, and they are away from any artificial structures which may influence turbidity.

The field experiment was designed to select particular waves, then simultaneously sample suspended sediment, measure wave process parameters and record sampling positions with respect to the beach profile, and photograph the sampling sequence. A 4 to 6 man field team was required to coordinate all of these functions.

A range was established at each experiment site and periodically surveyed to the low tide breaker line to calculate the beach slope at each sampling point. Fiberglass stakes were set throughout the surf

zone as reference points for sampling location and wave position. In addition, they provided convenient ranges to monitor longshore currents at the surface by means of slightly buoyant floats. Each suspended sediment sample was positioned relative to: 1) the bed, by means of the sampling apparatus; 2) a bench mark on land, by means of the reference stakes; 3) the wave breakpoint, by measuring the distance seaward or landward of each array; and 4) the time of passage of each wave sampled. Figure 18 is a sketch of a typical sampling arrangement.

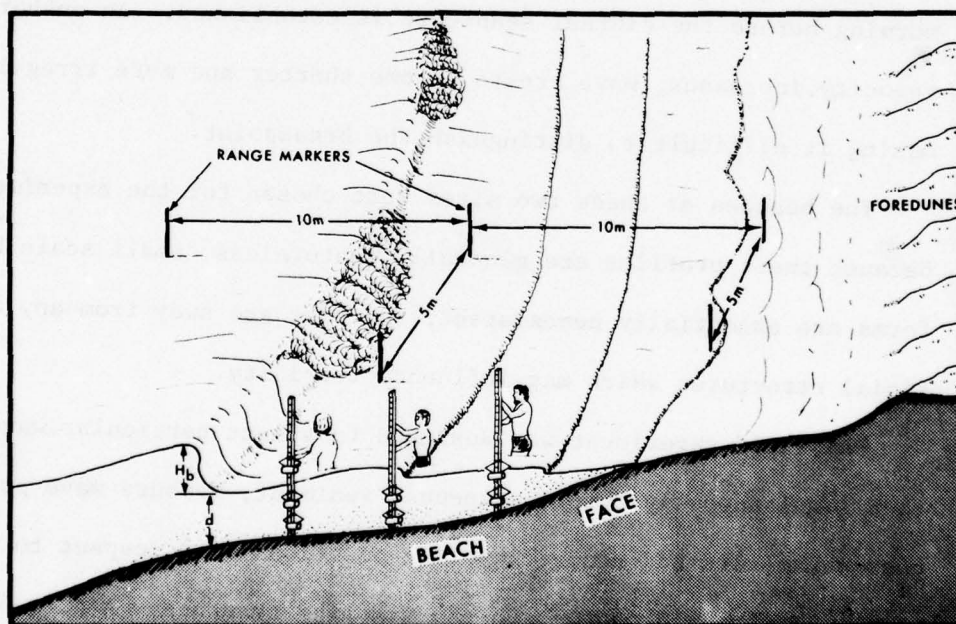


Figure 18. Sketch of the sampling arrangement showing samplers in place. Operators stood downdrift from sample point holding apparatus above bed until sampling instant. Range markers provided reference marks to locate sampling positions, measure longshore current velocity and distances from the breakpoint.

Suspended Sediment Measurements

Suspended sediment samples were collected and recorded by ARRAY. An array consists of all samples trapped simultaneously by one sampling apparatus. In most instances, the number of samples per array was 3, located at 10, 30 and 60 cm above the bed. As many as three arrays, with up to 10 water samples, were collected in each wave to for a SERIES. Thus, cataloguing each sample run, a SERIES designating the wave selected for sampling was recorded. Each SERIES contained from 1 to 3 individual vertical ARRAYS, and each ARRAY contained from 1 to 5 water samples. Samples were classified by their position in the ARRAY. The lowermost sample centered at 10 cm above the bed was designated SS10; the one at 30 cm was SS30, and so on through SS60, and SS100.

The samplers are "rigged" for operation according to the water depths. When small waves are sampled, only the lowermost bottles are mounted to the support pole. Figure 19 shows the bottle doors being cocked for sampling on the beach. Rigged samplers are then carried into the surf by each operator (Fig. 20).

Positioning in the surf is determined with respect to the mean breaker line, using reference stakes as guides. Similar to a surfer positioning himself to ride a wave, each operator positions the sampler at a predetermined distance from the breaker line. In general, the seaward-most operator controls the positioning and spacing between arrays and selects the wave. A fourth person is positioned downdrift and slightly landward of each operator to take a rapid sequence of photographs of the wave sampled. Additional personnel are positioned to measure the breaker height, wave period



Figure 19. Photo of simultaneous water samplers being rigged for operation at station Cal. Collecting bottle doors are held open by the trigger assembly (see also Fig. 4).



Figure 20. Photo of simultaneous water sampler ready for sampling 5 positions above the bed. Total weight with 4 empty bottles is 10.5 kg.

and surface longshore current velocity.

Based on tests conducted during earlier field experiments, it was determined that the most consistent results are obtained when the operator stands down-drift of the sampler, faces alongshore, and holds the apparatus away from his body in a vertical position several centimeters above the bed. At the desired sampling instant, the operator steps forward thrusting the device away from his body and into the bed. As the footpad at the heel of the sampler depresses on striking the bed, each water bottle closes automatically.

For this experiment, each array was collected approximately 2 seconds after passage of the wave bore by the sampler. Results from Kana (1977) and Brenninkmeyer (1976b) indicate that, in general, the maximum concentration at a point occurs after the passage of a wave. Thus, the intent of this procedure was to sample at the probable time of maximum concentration. With three operators, it was possible to follow one wave toward shore and, to some extent, determine the change in concentration across the surf zone.

Approximately 35 waves were sampled with multiple arrays positioned along individual wave crests in order to determine the range of variability of concentration at analogous positions in the breaker. Closest spacing of the samplers was 3 meters.

A typical photographic sequence of the sampling procedure is shown in Figure 21. Appendix A contains a photo atlas of some of the waves sampled categorized by breaker type and height.

After the water samples are trapped in each collecting bottle, the apparatus is carried ashore for transfer of the suspended sediment samples to holding jars. The photo sequence in Figure 22 shows

Figure 21. Four successive photographs of one of the breaking waves sampled at 3 positions inside the surf zone. Arrays of suspended sediment samples taken 3 m, 7 m, and 10 m landward of the breakpoint. Concentrations are listed below.

- A. Wave beginning to break 3 m seaward of sampler operator #1. $H_b = 90$ cm; $d = 95$ cm; $T = 8$ s; $V = 0$ cm/s; $m = 0.011$; breaker type: spilling; Time = 0 s.
- B. Wave fully broken, bore at Operator #1. Bore height = 70 cm; Depth under bore = 90 cm; Time ~ 1 s.
- C. Bore approaching operator #2. Bore height = 60 cm; Depth = 65 cm; Time ~ $2\frac{1}{2}$ s. Just before sampling instant array #1 (seawardmost).
- D. Bore at operator #3. Bore height = 55 cm; Depth = 60 cm; Time ~ 4 s. Just before sampling instant Array #2.

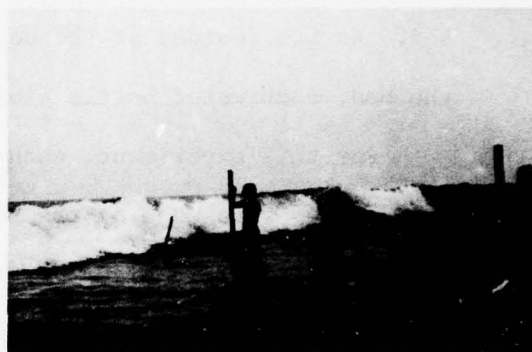
Results:

Array #	Elev. above bed (cm)	Conc. (gm/l)
1	60	.135
	30	.256
	10	.329
2	30	.261
	10	.234
3	30	.170
	10	.231

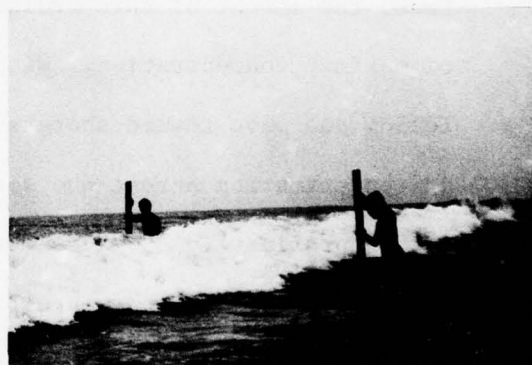
A.



B.



C.



D.



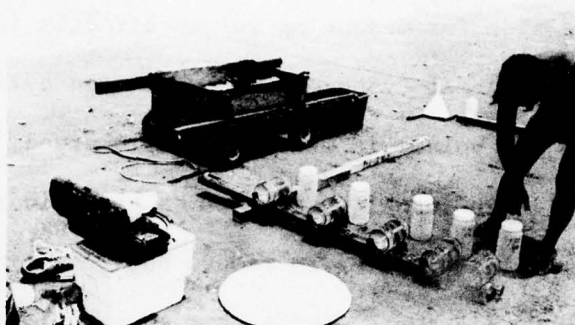


Figure 22a. Array of 5 samples ready for transfer to 2 l. Nalgene holding jars. Sampler bottles are calibrated to measure volume collected.



Figure 22b. Individual samples are opened into a large plastic funnel inserted in a holding jar.



Figure 22c. Sampler bottle is rinsed into the jars with distilled or deionized water to remove any particulates left on the bottle.

the method of transfer. Each collecting bottle has a calibrated scale mounted along the side for measuring volume directly in the field. This eliminates the normal transfer into graduated cylinders before filtering. After the volume is measured and recorded, each sample is emptied through a large funnel into a 2-l. Nalgene wide/mouth jar, then rinsed with distilled water to remove any remaining sediment. The jars are stored in a custom built cart then pulled 2 miles along the beach and ferried to a nearby lab for analysis (Fig. 23.).



Figure 23. Custom beach cart designed by D. Domeracki used to haul samples and equipment 2 miles down the beach for ferrying to a nearby lab. Brackets at the sides hold sampler poles, profile rods, range markers. Along the open beach, sand is hard-packed, making it relatively easy for 1 person to haul the 200 + kg weight. This is not the case, however, near the inlet where the sand is almost thixotropic.

The number of samples collected each day was restricted by the number of holding jars available and stamina of the operators to pull the load down the beach. Generally, 75 to 100 samples were collected massing up to 250 kg, including samplers and support equipment.

The following surf parameters were measured for each sample array:

1. Breaker height (H_b) and water depth at breaking (d) - visually, by means of a graduate staff held in place at the breakpoint and checked by scaling photographs taken during sampling. Limit of error ± 10 cm.

2. Bore height and depth under bore at each sample array - by the same method listed above.

3. Breaker type - qualitatively, by visual observations in the field, verified by photos taken while sampling and checked against various breaker-type parameters (e.g. Galvin, 1968; Battjes, 1974).

4. Wave period (T) - by averaging the time of travel between the wave crest prior to, and the crest following, the wave sampled. Limit of error ± 1 second.

5. Surface longshore current velocity and direction at mid-surf position (V) - by timing the travel of small floats between range markers set 10 m apart in the alongshore direction.

6. Breaker angle (α_b) - visually, by means of a protractor, sighting the acute angle between wave crest and shoreline. Limit of error ± 2 degrees.

7. Wind velocity and direction - by means of a hand-held anemometer.

Beach Surveying, Sample Positioning and Identification

Beach profiles at each station were measured approximately every 5 days during the experiment to provide information on average beach slope (m) and local slopes (s) at each sample array. Arrays were positioned along the profile with respect to distance seaward of a shore benchmark and distance seaward or landward of the wave breakpoint. Sample time with respect to passage of the bore by each array was estimated to the nearest second.

Other variables recorded included:

1. Station, date and time;
2. Tide from prediction tables;
3. Operator for each array; and
4. Orientation of the series of arrays (either perpendicular or parallel to wave crests).

An example data sheet including reduced concentration data is given in Figure 24.

Using up to 3 samplers in each wave, over 230 individual waves were sampled during the experiment, yielding approximately 1500 concentration values. Of these, over 1000 samples were collected at the lowermost positions 10 and 30 cm above the bed,

Table 1 summarizes the range of surf conditions and number of samples collected by wave type and wave height.

Laboratory Procedures

Suspended sediment samples. - In the lab, samples were filtered, using standard filtering apparatus and techniques to retain the suspended sediment. The filtering system used consisted of a Millepore vacuum pump, ballast jug and several manifolds of Millepore filtering flasks (Figures 25 and 26). Up to 20 samples could be filtered at the same time. Millepore 1.2 μ , 45 mm diameter cellulose filters were used and stored in locking petri dishes.

Since suspended sediment concentrations can be high in the surf zone, it is necessary to preweigh the filter together with the petri dish to allow for overflow of sediment on the filter. In cases where concentration exceeds 5 g/l, several weighed petri dishes and filters may be required to retain all of the sample. This is easily accom-

SURF ZONE SUSPENDED SEDIMENT DATA FORM

Station CA1 Date 10 JULY 1977
 Series # 60711 Time 1045

Wave Parameters: H_b (cm.) 95 Depth Breaking 80 Distance to Front Stk. Array 1 - 95m
Array 2 - 90m
 α (°) 3° Right Breaker Type Plunging NOTES: Photos - Roll PE 18 Fr 13-15
 T (sec.) 6.5 L.S. Current 5m @ 15s, 18s, 19s Sup 1 - wave cresting
 2 - Base at array #1
 3 - Base at array #2
 SS Conc. NOTES

Notes	Sampling Bottle #	Collecting Bottle #	Volume (ml.)	Height abv. Bot.	Filter #	Filter Wt.	Filter + Sed.	Sed. Wt.	
Array #1	5	27	Full 2030	60 cm	A167	6.273	6.898	.625	.315
	8	58	Full 1985	30	A234	6.350	8.567	2.217	1.112
	3	13	Full 1997	10	A305	6.298	11.648	5.350	2.675
Array #2	12	95	Full 1977	30	A216	6.300	9.423	3.123	1.564
	10	78	Full 1995	10	A217	6.150	12.782	6.632	3.317

oceanic - 1000
 3m from Breakpoint
 oceanic - 1000
 8m from B.P.

Figure 24. Example field data form.

Table 1. Distribution of samples by wave height and breaker type.

Breaker height Hb (cm)	Breaker Type - # Waves Sampled			#Vertical arrays of* susp. sediment samples
	Spilling	Spill/Plunge	Plunging	
0-35	5	4	2	29
35-45	7	1	3	32
45-55	15	2	5	56
55-65	12	4	1	46
65-75	15	8	10	92
75-85	29	6	7	118
85-95	13	11	18	115
95-105	8	9	5	63
105-115	10	3	2	45
115-125	6	2	1	27
125-160	11	0	0	33
Totals	131	50	54	656

* 3 vertical arrays of 1 to 4 samples each were collected in each wave to yield a total of approximately 1500 concentration values.

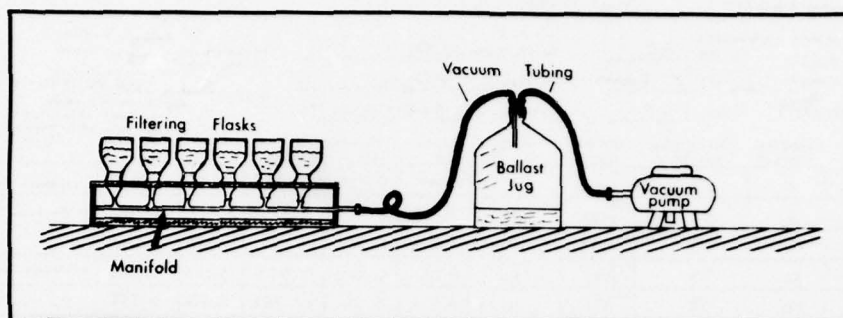


Figure 25. Sketch of the filtering apparatus with the three principal components. Use of several manifolds allowed 20 samples to be processed simultaneously. Filtering times for 2-l. samples ranged from 1 hour to 4 days.



Figure 26. Photo of filtering set up in a temporary field laboratory located at Price Inlet. Rubber "policeman" in Walter J. Sexton's right hand is used to wipe sediment down rim of filtering flasks.

plished by spooning out the excess sediment from the filter flask and transferring it to a second filter. The combined weights of sediment on the two filters are calculated to determine concentration.

Filtered samples were rinsed 3 times with distilled water to remove salt, then stored for shipment to the Department of Geology. The final laboratory analysis entailed drying the filters and sediment at 70°C, then weighing the petri dish, sediment and filter. All concentrations were determined as a weight per unit volume (grams/liter).

Size analysis of suspended sediment. - Approximately 140 samples from the present experiment were analyzed for size distribution using the Hydraulic Equivalent Sediment Analyzer (HESA) developed by Anan (1972) for the Coastal Research Division, Department of Geology. An additional 75 suspended sediment and 45 beach samples collected in a 1975 experiment were analyzed to augment these data. Grain size frequencies were computed and graphed by a Hewlett Packard 9825A mini-computer and 9872A plotter interfaced with the HESA.

Various sample masses were tested to determine the reproducibility of the system. It was found that a 2-3 gram sample was optimal producing almost identical curves for a given sample and means within 2 tenths of a phi unit. Samples of 1 gram varied as much as one-half phi unit. Since most of the suspended sediment samples had relatively low concentrations, only those samples containing at least 1 gram could be sized. Representative frequency plots are given in the section on results.

Computer Coding the Data

The entire data set for this experiment was coded for statistical analysis on the IBM 370/168 computer at the University of South Carolina. The principle data set included all suspended sediment, wave process, location and slope information. A second data set was used for size statistics.

The data were keypunched onto IBM cards, then transferred to internal mass storage on the master disk for more convenient use. Statistical analyses were performed by the preprogrammed Statistical Analysis System (SAS76) and the Statistical Package for the Social Sciences (SPSS). After considerable experimentation, it was found that the SAS76 system was more convenient to use.

Most of the data plots presented in the next chapter were generated by the computer, then redrafted to a more presentable format.

Each sample array is listed as an observation. A typical observation has 32 variables, including sample identifiers, corresponding wave process values, and suspended sediment concentrations at each reference level. Twelve additional variables are computer generated from the field data, including log transformations of concentration and breaker type parameters.

Appendix B contains a listing of computer variable names, a sample SAS76 program, and a data printout for selected observations and variables.

Rejection of Data

A prerequisite to any statistical analysis is the determination of sample distribution. The principle variables were tested for normal distribution by means of normal probability plots. Examples for the independent variables wave height, wave period, ratio d_b/H_b , long-shore current velocity, wind velocity, and beach slope are given in Figure 27. Note that each of the data points represents multiple observations. In general, it can be seen that each variable is approximately normally distributed. Beach slope shows, perhaps, the poorest fit. Relatively few outliers (circled on each graph) occur in each sample population.

The dependent variables of suspended sediment concentration (SS10, SS30, SS60 and SS100) were similarly tested, but found to be non-normally distributed. Therefore, various transformations were performed to determine if some function of concentration is normally distributed. A test against the Log_{10} of concentration yielded a reasonable fit to the normal probability plot and, consequently, this transformation was used in the following analyses. Figure 28 gives cumulative frequency curves for SS10 and SS30 plotted on a log scale. A similar relation exists for SS60 and SS100 (not shown). Note concentrations decrease with elevation above the bed and have the widest range of values at the lowermost sample (SS10).

For the present analysis, certain observations were rejected on the basis of the following criteria:

1. Sample array was obtained seaward of the breakpoint or more than 12 m landward of the breakpoint. This was to limit the data to a narrower portion of the surf zone directly influenced by the pri-

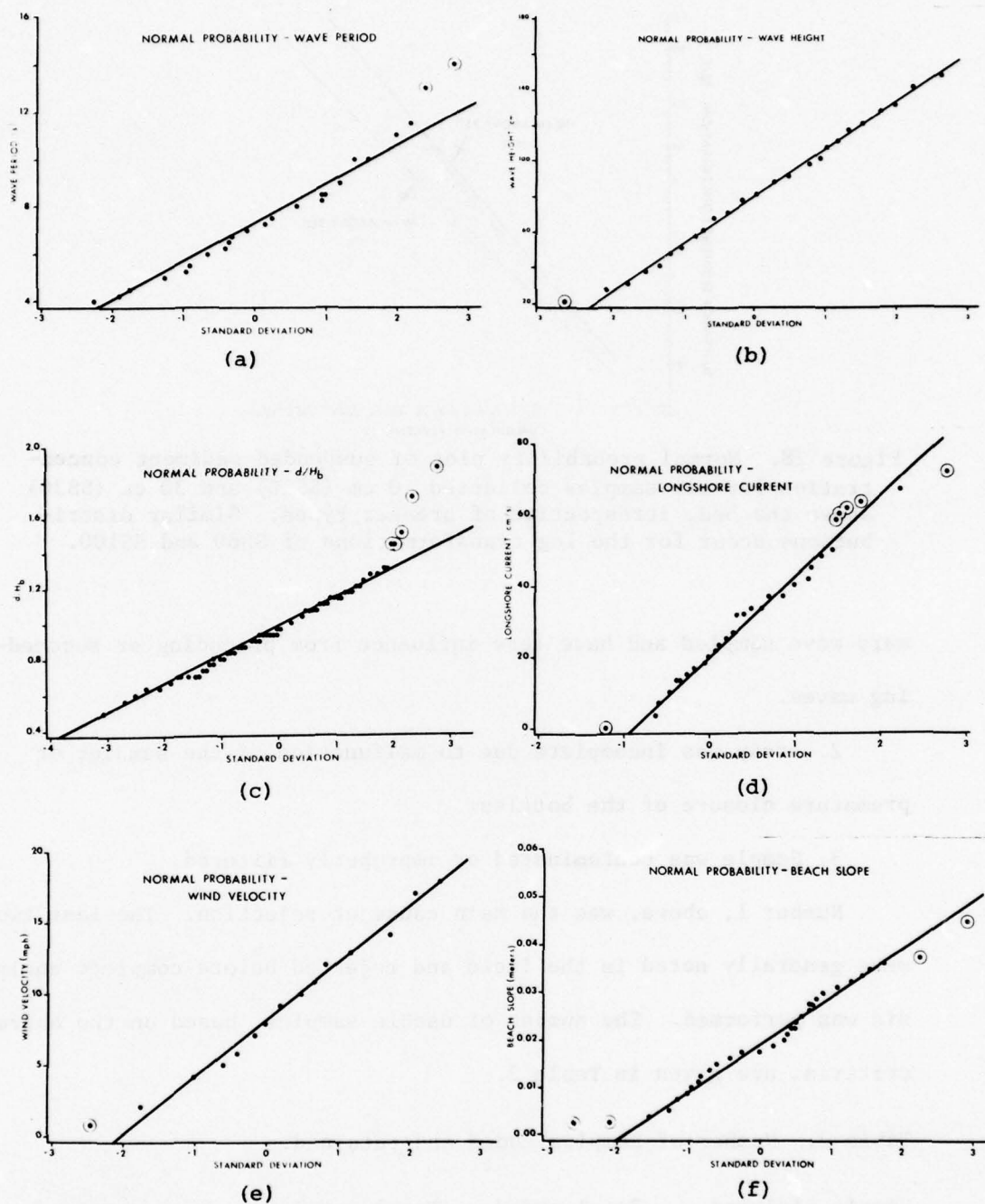


Figure 27. Normal probability distribution for the independent variables wave height (a), wave period (b), ratio d/H_b (c), longshore current velocity (d), wind velocity (e), and beach slope (f). Each data point represents multiple observations.

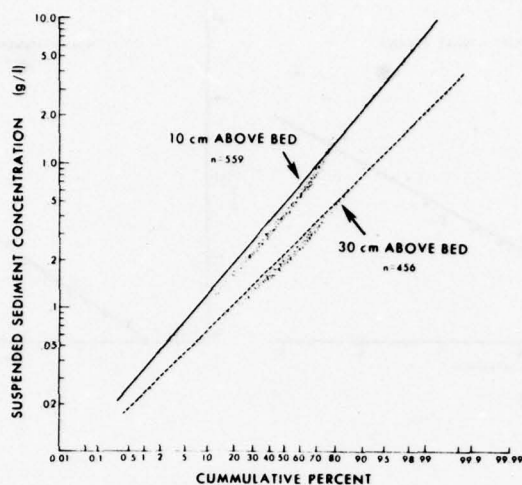


Figure 28. Normal probability plot of suspended sediment concentration for all samples collected 10 cm (SS10) and 30 cm (SS30) above the bed, irrespective of breaker types. Similar distributions occur for the log transformations of SS60 and SS100.

mary wave sampled and have less influence from preceding or succeeding waves.

2. Array was incomplete due to malfunction of the sampler or premature closure of the bottles.

3. Sample was contaminated or improperly filtered.

Number 1, above, was the main cause of rejection. The last two were generally noted in the field and rejected before complete analysis was performed. The number of usable samples, based on the above criteria, are given in Table 2.

Table 2. Number of samples coded and retained.

<u>Sample position</u>	<u>Total coded</u>	<u>Total retained</u>	<u>% retained</u>
SS10	559	405	72.5
SS30	456	343	75.2
SS60	262	175	66.8
SS100	14	11	78.6

RESULTS AND DISCUSSION OF THE DATA

Introduction

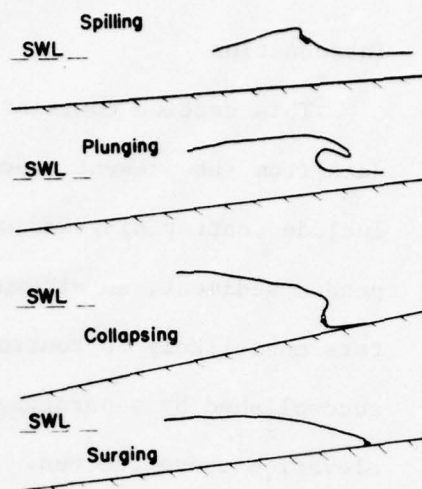
This section contains a reasonably detailed description of the data from the present experiment. Since it would be impractical to include scatter diagrams of every independent variable against suspended sediment, an attempt is made to sort the data by the parameters most likely to control concentration. Data sorting was first accomplished by separating samples into the natural categories by elevation above the bed. Next, breaker type parameters were tested against concentration to sort the data into subsets, and finally, the independent variables (wave process and location parameters) were tested against concentration for each subset.

Based on previous results and the inherent variability of surf zone suspensions, the tack employed herein was to sort the data by the independent variables (e.g. wave parameters), determine mean values for the dependent variables (SS10, SS30, etc.), then test the correlation of means against each parameter. By grouping instantaneous suspended sediment samples collected under similar conditions (e.g. with respect to wave height), it is possible to interpret much of the variability observed. Where mean values plotted on the following diagrams represent more than 6 observations, the limits to ± 1 standard deviation are indicated.

Overall Means

Values of suspended sediment concentration were naturally divided by sample elevation above the bed (SS10, SS30, SS60 and SS100). Similar to an earlier experiment (Kana, 1977), the data were sorted according to breaker type determined visually in the field using Galvin's classification (Fig. 29). Examples of the principle wave types

Figure 29. Primary members of the commonly used breaker classification of Galvin (1968). Sketches are typical cross sections at the breakpoint. In general, as beach slope increases, breaker type varies from spilling to surging (From Galvin, 1972).



sampled are given in Figure 30. Galvin's classification actually represents certain members of a continuous spectrum of breaker types. Along South Carolina beaches, spilling and plunging waves are most common, but an intermediate type is often observed which exhibits characteristics of each type. For example, viewing along a breaking wave crest, one often sees portions of a wave spilling early, then a section plunging closer to shore (Fig. 30b). In some cases, the percentage of the wave spilling or plunging can be estimated and coded appropriately (e.g. 50/50 for 50% spilling and 50% plunging). Waves that were not clearly spilling or plunging were designated under the category, TRANSITION. Non-breaking waves were so classified and eliminated from the present analyses. In summary, three categories of waves were used: SPILLING, TRANSITION and PLUNGING.

Figure 31 gives overall mean suspended sediment concentration by elevation above the bed and visually classified breaker type. The breaker type parameters B_b and d/H_b listed on the figure are discussed in the next section. Note that plunging type waves sus-

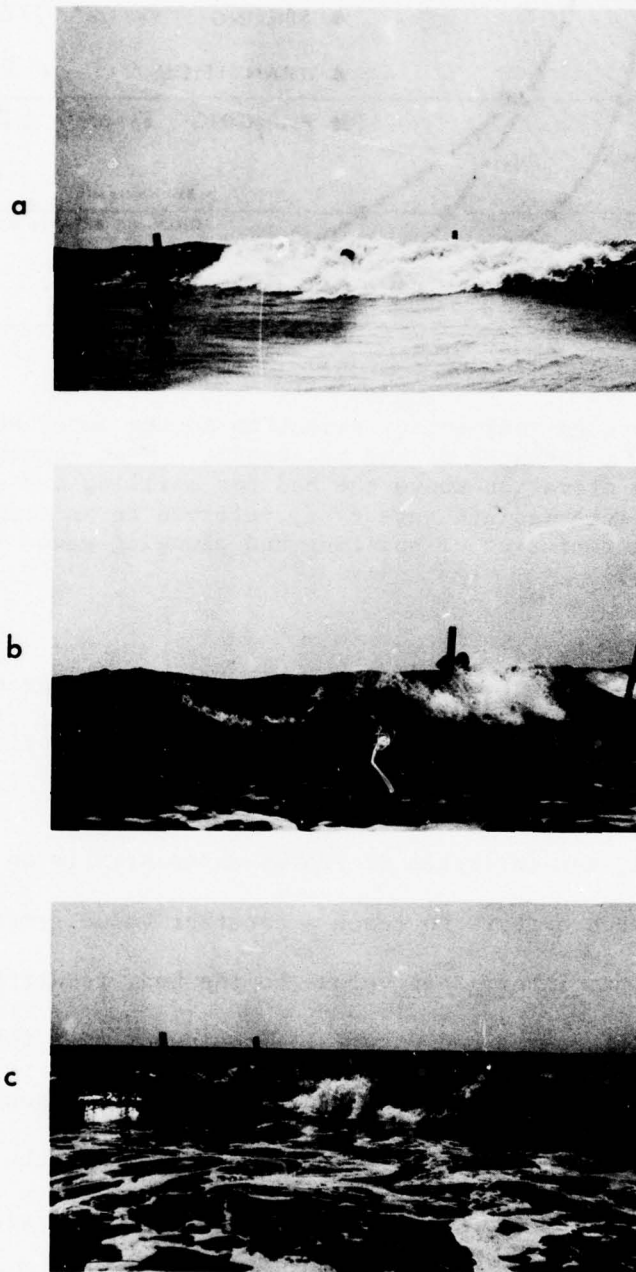


Figure 30. Breaker types most commonly observed near Price Inlet, S.C., including spilling (a), transition (b) and plunging (c). Transition waves exhibit characteristics of both primary wave types. In general, the ratio breaker depth to breaker height, (d_b/H_b) , decreases from spilling to plunging waves with a value of approximately 1.0 in transition waves.

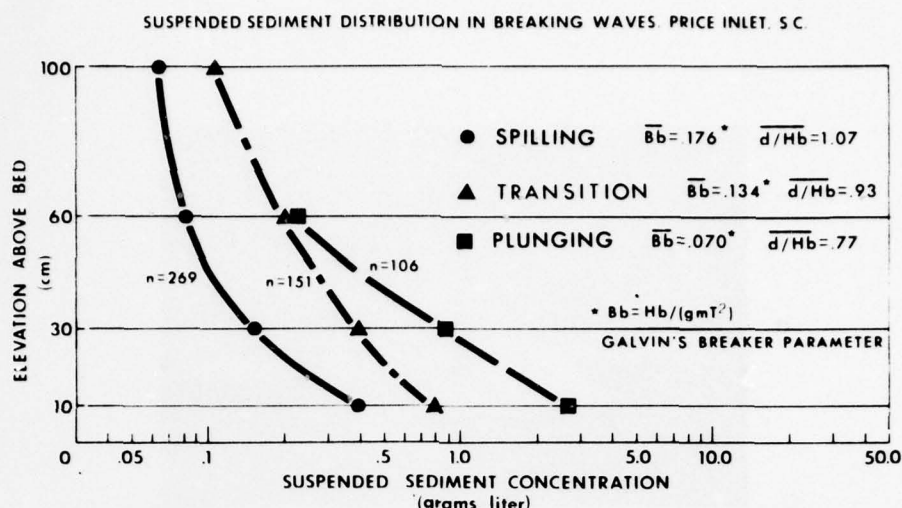


Figure 31. Suspended sediment distribution in the outer surf zone between 1 and 10 m landward of the breakpoint. Mean concentrations are plotted by elevation above the bed for spilling and plunging type breakers. An intermediate wave type, referred to as transition, exhibiting characteristics of spilling and plunging waves is also plotted (triangles).

pend as much as one order more sediment than spilling waves. The curve for transition waves, of course, falls between the two primary wave types.

In general, concentration decreases exponentially up to 60 cm above the bed, then appears to reach a constant value, reflecting the type of suspension: intermittent close to the bed, resulting from periodic bursts of coarse bed material; continuous near the surface, due to the dominance of suspended wash load. Based on separations the sand-silt boundary (sediment diameter = 0.063 mm), the wash load of silt and clay sized sediment averaged less than 0.05 g/l in each sample. Consequently, samples higher in the water column but lower in concentration have the highest percentage of fines. This will be discussed in more detail in the section on size distribution.

Table 3 contains a listing of mean values of selected variables sorted by date. A more complete data listing is given in Appendix B.

Table 3. MEAN VALUES FOR SELECTED SUSPENDED SEDIMENT AND WAVE PROCESS VARIABLES BY DATE.

Date (1977)	SS10 (g/l)	SS30 (g/l)	d_b/H_b	Predominant Breaker Type	Beach Slope	H_b (cm)	d_b (cm)	T (s)	V (cm/s)	α_b (°)	Wind (mph)	# SS Arrays
11 June	.189	.058	1.13	spilling	.018	61	69	7.5	3	0	8.8	39
12 June	1.151	.645	1.08	spilling	.019	67	75	5.4	32	6	10.7	26
13 June	.449	.263	.85	plunging	.016	75	64	6.6	16	5	3.1	36
14 June	.627	.308	1.01	transition	.017	90	90	6.9	34	4	11.9	21
15 June	2.155	1.035	.94	plunging	.011	61	57	8.4	8	4	4.1	37
16 June	.373	.169	1.21	spilling	.012	54	62	7.4	40	9	10.6	20
18 June	.436	.096	1.14	spilling	.019	40	45	8.1	0	0	9.2	34
19 June	.298	.184	1.05	spilling	.022	59	60	4.8	43	9	12.7	21
20 June	.269	.133	1.00	transition	.018	98	98	6.3	34	6	7.0	27
21 June	1.287	.271	.85	plunging	.012	50	45	6.1	12	0	8.9	16
28 June	1.608	.243	.92	plunging	.023	62	58	7.5	5	0	5.3	42
29 June	3.173	.878	.72	plunging	.023	84	60	7.7	39	1	7.5	32
30 June	.912	.506	.87	plunging	.018	81	70	7.4	26	4	7.0	19
1 July	.849	.451	.97	transition	.026	87	84	7.2	44	4	15.1	27
2 July	1.046	.396	.85	plunging	.023	90	77	9.6	14	3	3.3	35
4 July	.456	.207	.98	transition	.019	92	89	8.1	20	3	6.0	33
5 July	.358	.281	.93	plunging	.024	94	88	8.0	0	0	4.0	20
6 July	.301	.182	.97	transition	.018	121	117	4.9	24	3	11.4	29
7 July	1.077	.338	.99	transition	.023	109	108	6.2	34	5	9.0	23
8 July	.587	.330	1.01	transition	.018	105	107	5.2	61	3	13.0	19
9 July	.477	.462	.90	plunging	.029	94	85	6.0	7	2	10.0	5

Tests of Concentration vs. Breaker Type Parameters

Introduction. - Since breaker type may be a very significant factor controlling the magnitude of intermittent suspensions in the surf zone, it is desirable to obtain relations which quantify wave variability. According to laboratory studies by Patrick and Wiegel (1954), and Galvin (1968, 1972), the way a wave breaks at the shore is primarily dependent on the parameters of beach slope (m), wave height (H_b), and wave steepness (H_b/L_o), where m is measured as the tangent of the acute angle between beach face and horizon, H is wave height measured from trough to crest, and L is wave length measured between successive crests, and the subscripts b and o refer to inshore breaking dimensions and offshore deepwater conditions, respectively. From Airy (1845), linear wave theory, L_o is related to wave period (T) by:

$$L_o = \frac{gT^2}{2\pi}, \quad (1)$$

where g is the acceleration of gravity. Thus, wave steepness is often given in terms of H and T since these parameters are more commonly measured.

The above relationships have allowed Galvin (1968) and Battjes (1974) to present two breaker type parameters based on wave steepness and beach slope. Galvin's dimensionless onshore parameter (B_b), utilizing surf zone wave measurements, is given by:

$$B_b = H_b / (gmT^2). \quad (2)$$

Data obtained on plane concrete laboratory beaches set at various slopes give the following transition values between wave types: 0.068 between spilling and plunging waves; 0.003 between plunging and surging waves.

Battjes (1974) gives a surf similarity parameter (ξ - Greek letter XI) defined by:

$$\xi = m/(H_b/L_o)^{1/2}. \quad (3)$$

A simple transformation gives this parameter in terms of wave period as:

$$\xi = m/(2\pi H_b/gT^2)^{1/2}. \quad (4)$$

Battjes' transition values between breaker types are numerically different, but equally comparable with Galvin's. Approximate values listed by Battjes (1974, p. 470) are: $\xi = 0.4$ between spilling and plunging; and $\xi = 2.0$ between plunging and collapsing.

Although B_b and ξ can be readily applied since the dependent variables H_b and T are easy to measure, and they appear to separate major wave types, it is the opinion of the writer that these parameters are not sensitive enough to distinguish among waves of a particular class at one beach. Experience has shown that on a given beach, most waves will fall within a small range of T , H and m values. B_b and ξ will vary only slightly. The main advantage of these parameters is in comparison of waves on different beaches. It is also the opinion of the writer that they lack the fundamental parameter, relative wave height, which should be considered in describing wave breaking.

Breaker types form a continuum ranging from spilling breakers on gentle slopes through plunging and collapsing on steeper slopes, then, finally, surging waves on very steep to vertical slopes (Figures 29 and 30). And as Galvin (1972) shows, for a given beach slope with a variation in wave height alongshore, breaker type changes as follows:

highest waves are generally spilling; higher intermediate waves are plunging; lower intermediate waves will tend to be collapsing, and the lowest waves will be surging. As beach slope and wave energy cause a variation in breaker type, relative wave height also changes.

Relative wave height is generally defined as the ratio H_b/d_b , where d_b is the depth at breaking. Munk (1949) was one of the first to recognize its importance in surf problems and used it as the fundamental parameter in his development of Boussinesq's (1872) solitary theory for the special case of shoaling water waves. Munk argues that wave length, a function of T , has little to do with the shape of waves near the breakpoint since waves in very shallow water commonly have long, flat troughs and steep, sharp crests. Solitary theory differs from linear theory by eliminating the dependence on T or wave steepness.

Another reason for retaining relative wave height is its variability at the breakpoint. Although McCowan (1894) demonstrated theoretically and Munk (1949) empirically that the maximum ratio of H_b/d_b at breaking should be 0.78, measurements by Ippen and Kulin (1955) indicate that breaking occurs over a range of H_b/d_b values greater than 0.78, depending on the slope. On steep slopes, Ippen and Kulin found that H_b/d_b may exceed 2.8. In the field, it's been noted by Galvin (1972) that collapsing waves on steep slopes break against the beach face where d_b is close to zero, and H_b/d_b will approach ∞ . Thus, as H_b/d_b at breaking increases, waves will pass through a continuum of breaker types starting with spilling at the lowest relative wave heights, then to plunging and collapsing at intermediate H_b/d_b values, and finally to surging at highest values.

In summary, for purposes of formulating a breaker type parameter,

not only H_b , T and m should be considered, but also breaker depth, d_b ¹.

Galvin's Parameter (B_b). - The dependent variables SS10, SS30 and SS60 were plotted against Galvin's inshore parameter (B_b), as a first attempt to quantify breaker type. In all cases, the scatter is considerable. A plot of SS10 vs. B_b for samples collected up to 12 m landward of the breakpoint is given in Figure 32. Each data point represents a mean concentration for a particular B_b value. Note that most of the data falls near the spilling-plunging transition value. Although the regression line is significant at the .04 level, it accounts for almost none of the variability in the data ($r^2 = .03$). Furthermore, the relatively flat slope of the regression line suggests that B_b is unsatisfactory as a predictor of concentration by wave type. Figure 33 shows calculated regressions of SS10, SS30 and SS60 on B_b , all of which are inadequate for distinguishing these data. The separation of the lines between SS10 and SS30 is not surprising given the exponential decay of concentration above the bed indicated in Figure 31.

Battjes' Parameter (ξ). - When plotted against mean concentration, Battjes' (1974) surf similarity parameter (ξ) provides similar results as Galvin's B_b . Figure 34 shows the scatter of SS10 values vs. ξ for samples collected within 12 m landward of the breakpoint. Although there is an apparent increase in concentration with increasing ξ , in agreement with the expected trend from spilling to plunging waves, the separation of wave type and concentration is poor. Further-

¹ In this discussion, relative wave height is H_b/d_b using the conventions of the theorists. However, for purposes of analysis, the commonly applied engineering inversion of this relation, d_b/H_b , will be used. McCowan's (1894) breaking criteria under this convention then becomes the oft-cited $d_b/H_b = 1.28$.

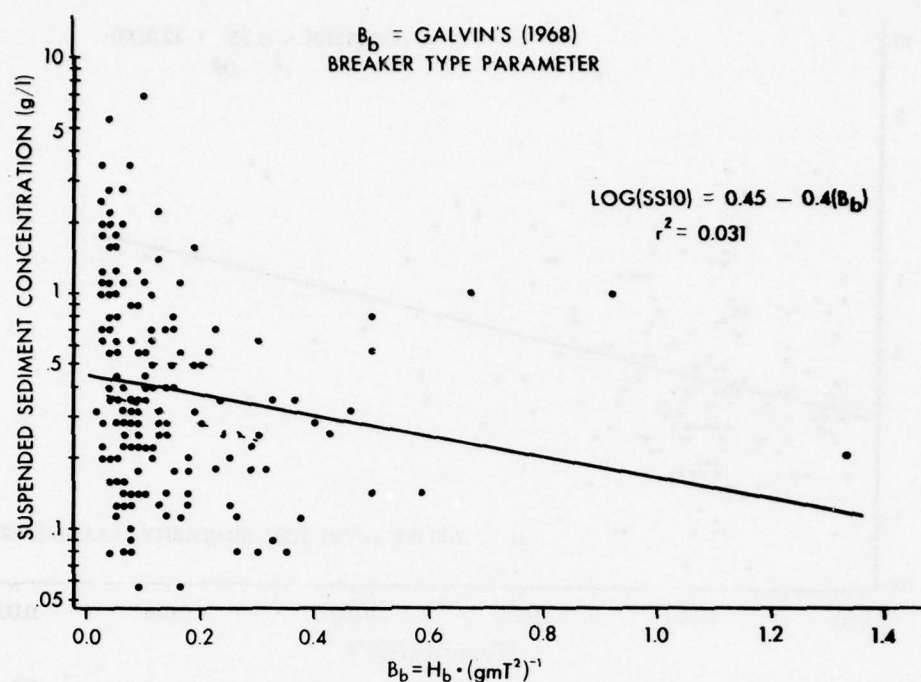


Figure 32. Scatter plot of mean SS10 vs. Galvin's inshore breaker type parameter (B_b). Data is concentrated near spilling/plunging transition value of .068. There is essentially no correlation between B_b and suspended sediment concentration for these data.

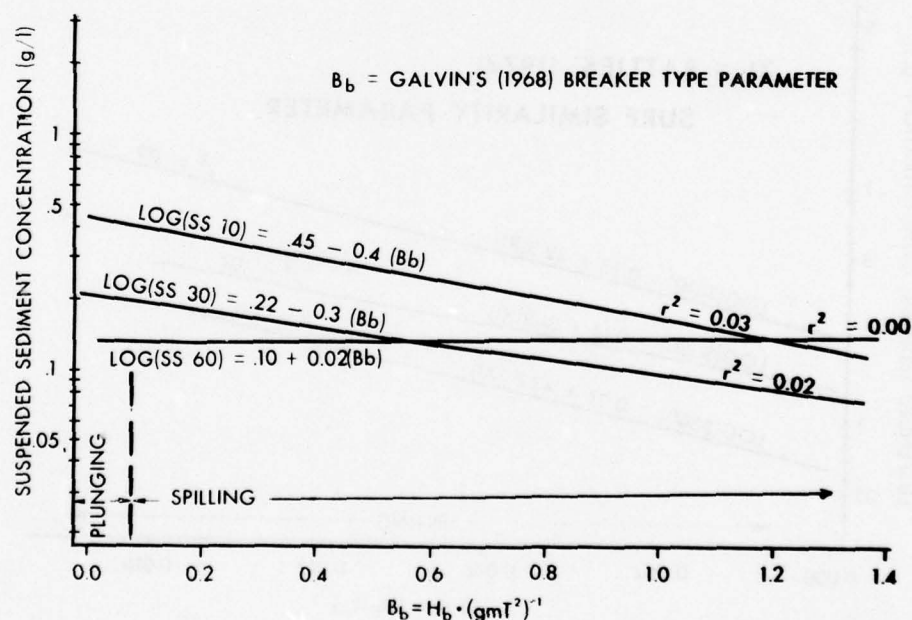


Figure 33. Linear regression models of SS10, SS30 and SS60 vs. B_b for suspended sediment samples within 12 m landward of the break-point. Miniscule correlation coefficients and flat slopes of curves indicate these models are inadequate for predicting concentration by this breaker type parameter.

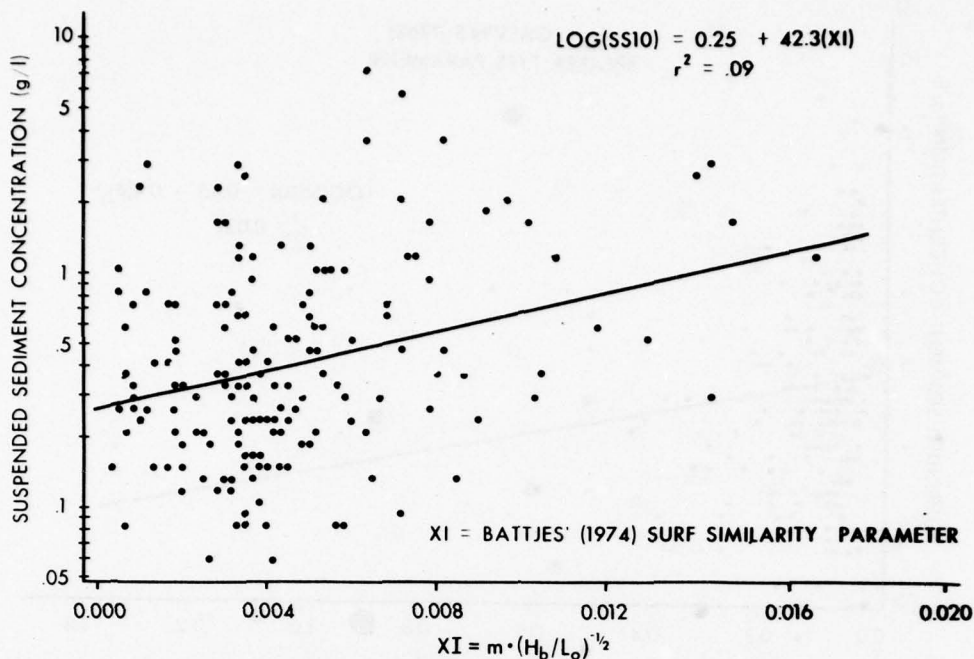


Figure 34. Mean SS10 vs. Battjes' surf similarity parameter, XI. All data fall within the spilling range given by Battjes (plunging waves are off the scale to the right). There is slightly improved correlation over Galvin's B_b ; however, less than 10% of the data are accounted for by the indicated regression line.

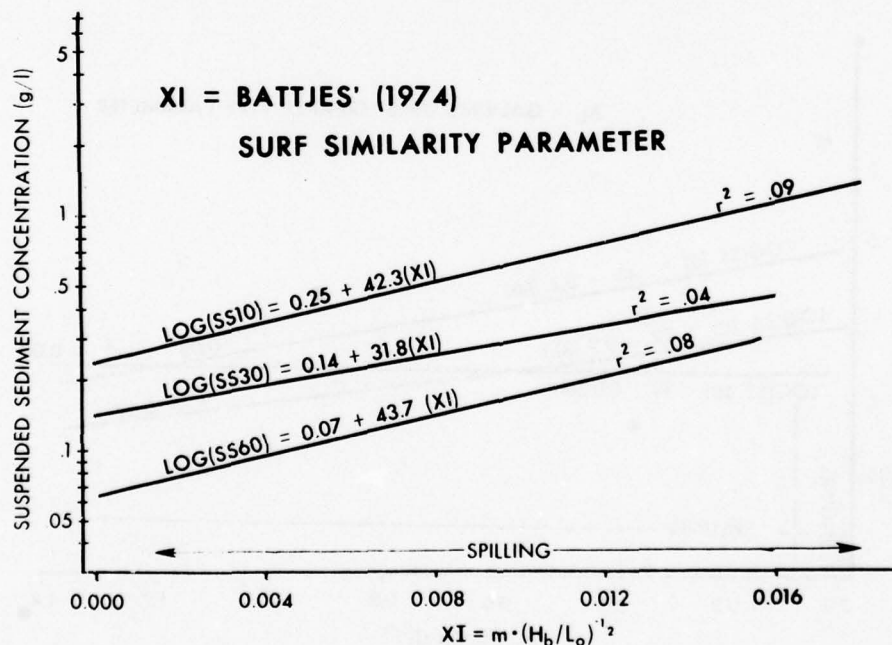


Figure 35. Linear regression models of SS10, SS30 and SS60 vs. XI for suspended sediment samples within 12 m landward of the breakpoint. All lines account for less than 10% of this variability and are unacceptable as a predictor of concentration.

more, all the data plot within the spilling range given by Battjes.

Figure 35 gives the probable regression lines for SS10, SS30 and SS60 against ξ . The separation by sample position is clear, but the regression lines account for less than 10% of the variability in the data.

Parameter BRKER. - Because of the poor fit of the data to published breaker type parameters, several alternative functions were calculated and tested. Although this was a hit or miss proposition in the beginning, since few of the independent variables correlated well with the data, it was apparent during field observations that relative wave height changed significantly from spilling to plunging waves. Therefore, a function was sought which included the ratio of wave height to breaker depth, or, using the engineer's convention, d_b/H_b . Another variable thought to be important is beach slope, m . So, various dimensionless quantities involving d_b/H_b and m were tested against the concentration data.

One such function was designated BRKER, after the computer coding. BRKER is given by:

$$\text{BRKER} = (1-m)^4 \cdot d_b/H_b \quad (5)$$

There were several reasons for retaining the form $(1-m)^4$. First, d_b/H_b ranges from .6 to 1.4 (plunging to spilling), while m ranges from 0.002 to 0.040 for these data. From a practical standpoint, it was preferable to obtain BRKER values of the order 1.0. This was accomplished by using $(1-m)$ instead of m . Furthermore, it was hypothesized that plunging breakers should occur on steeper slopes at lower d_b/H_b values. The function $(1-m)$ multiplied by d_b/H_b theo-

retically should increase the range of values corresponding to breaker types. As m increases and d_b/H_b decreases in plunging breakers, function BRKER will decrease. In spilling waves with small m and large d_b/H_b , the function stays large. Thus, there should be a better separation between breaker types using this form. The exponent was added simply to increase the range of values of $(1-m)$.

Comparing function BRKER to visual typing of waves indicated the following range of values correspond to each breaker type:

Spilling - BRKER > 1.07
 Transition - 0.91 - 1.07
 Plunging - BRKER < 0.91.

The relation between BRKER and SS10, SS30 and SS60 are given in Figure 36. It is evident that, for these data, this function yields a much better separation of concentration values by wave type than Galvin's or Battjes' parameters. Each data point represents the mean of 2 to 5 samples taken under similar conditions with respect to m and d_b/H_b . In addition, the regression lines yield values of concentration very close to those obtained using a visual wave classification given in Figure 31. For example, for SS10 (Fig. 36a), this function gives mean concentration values for plunging waves in the approximate range 0.4 to 2.0 and spilling from 0.05 to 0.2, the order of magnitude difference previously noted. Based on the relatively high number of mean data points for each sample, the indicated regressions are statistically significant at the .01 level and account for as high as 41% (SS10) of the variability in the data. There is still a lot of variation to account for, but function BRKER appears to separate wave types observed at Price Inlet better than the two published breaker parameters.

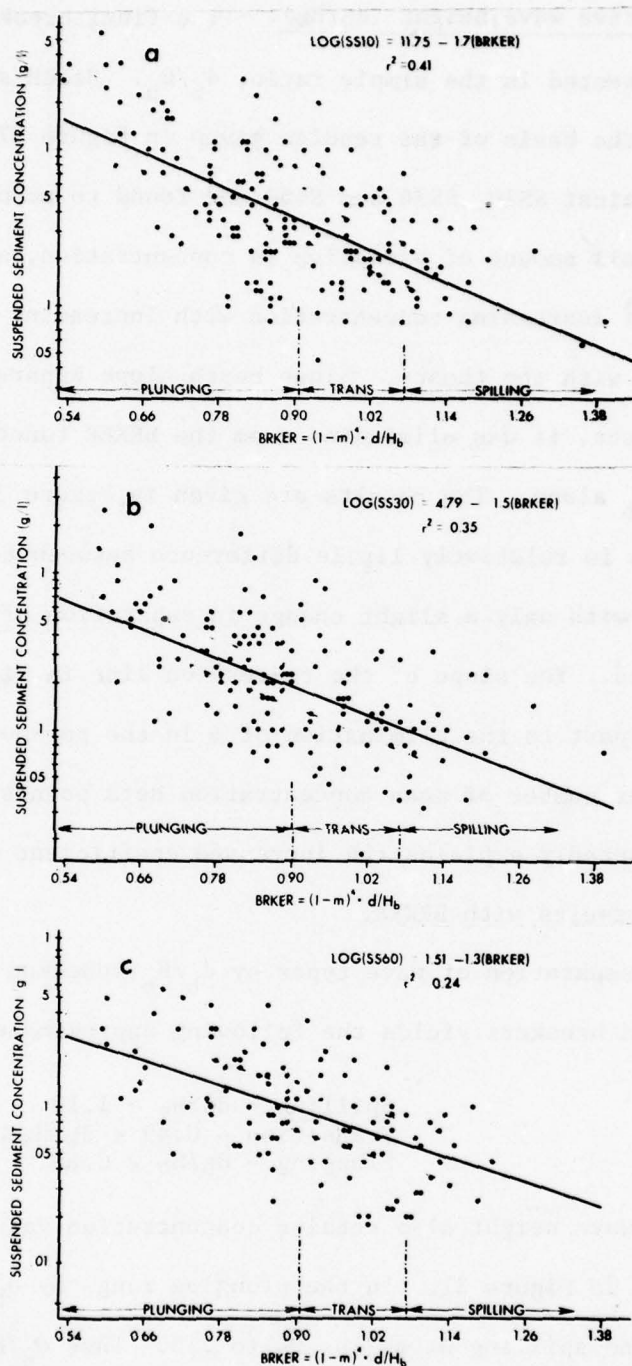


Figure 36. Linear models of SS10 (a) and SS30 (b) and SS60 (c) vs. parameter BRKER, showing much improved correlation. This function yields mean suspended sediment concentrations for SS10 in the range 0.4 to 2.0 for plunging waves and 0.05 to 0.2 for spilling, corresponding to the range of values and order of magnitude difference obtained using a visual breaker classification (Fig. 31).

Relative wave height (d_b/H_b). - The final breaker type parameter to be presented is the simple ratio, d_b/H_b . Beach slope, m , is eliminated on the basis of the results given in Figure 37. Beach slope was tested against SS10, SS30 and SS60 and found to account for a relatively small amount of variation in concentration, although there exists a trend of increasing concentration with increasing beach slope in agreement with the theory. Since beach slope apparently has a rather small effect, it was eliminated from the BRKER function in order to test d_b/H_b alone. The results are given in Figure 38.

There is relatively little difference between the plots of d_b/H_b and BRKER with only a slight change in separation of the concentration values. The slope of the regression line in Figure 38 is steeper due in part to the elimination of m in the parameter, as well as the smaller number of mean concentration data points for d_b/H_b . This undoubtedly explains the increased coefficient of determination over the results with BRKER.

The separation of wave types by d_b/H_b , checked against the visually typed breakers yields the following approximate ranges.

Spilling - $d_b/H_b > 1.10$
 Transition - $0.89 < d_b/H_b < 1.10$
 Plunging - $d_b/H_b < 0.89$

Relative wave height also retains concentration values in the ranges indicated in Figure 31. In the plunging range by d_b/H_b , SS10 is .5 to 2.0 g/l and spilling waves is .04 to .15. Thus d_b/H_b appears to be a fairly good indicator of breaker types for these data.

Sorting the data by d_b/H_b . - From the previous results, it was deemed possible to classify wave types on the basis of the quantifiable variable, d_b/H_b . Despite a significant amount of data variation

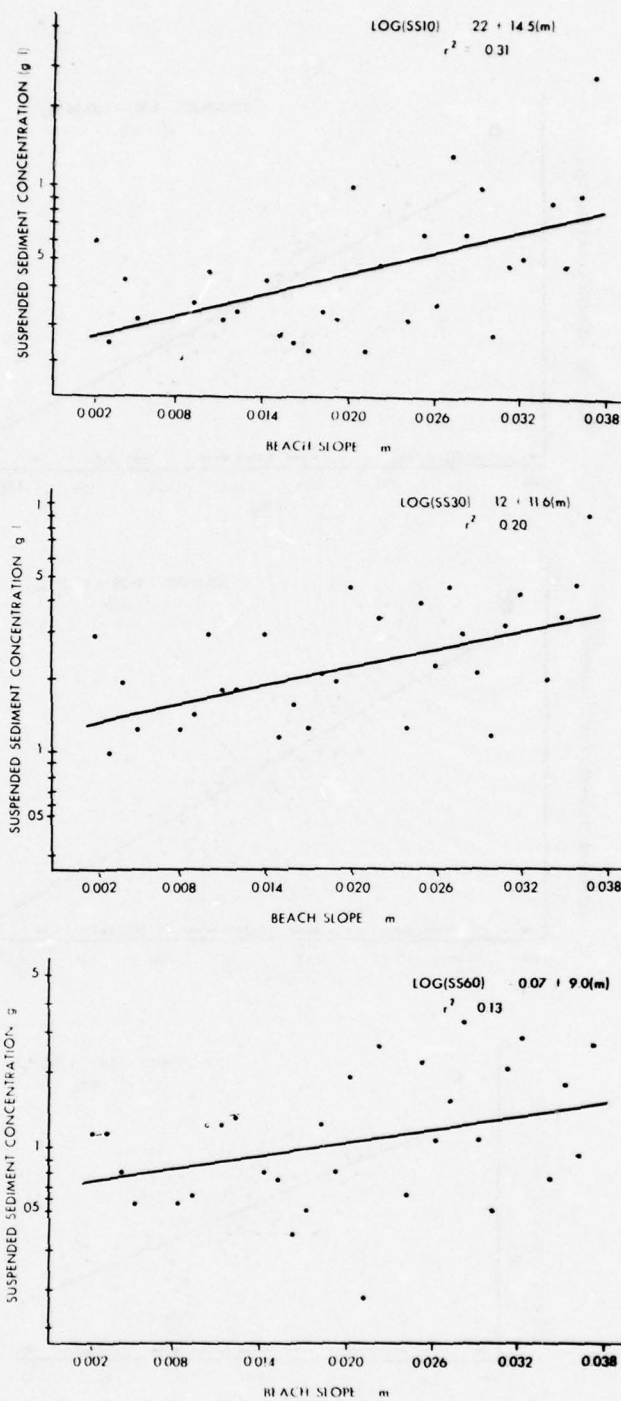


Figure 37. Mean SS10 (a), SS30 (b) and SS60 (c) vs. beach slope (m) showing general increase in concentration with increasing slope. Indicated regression lines account for up to 31% of the variability in mean concentration.

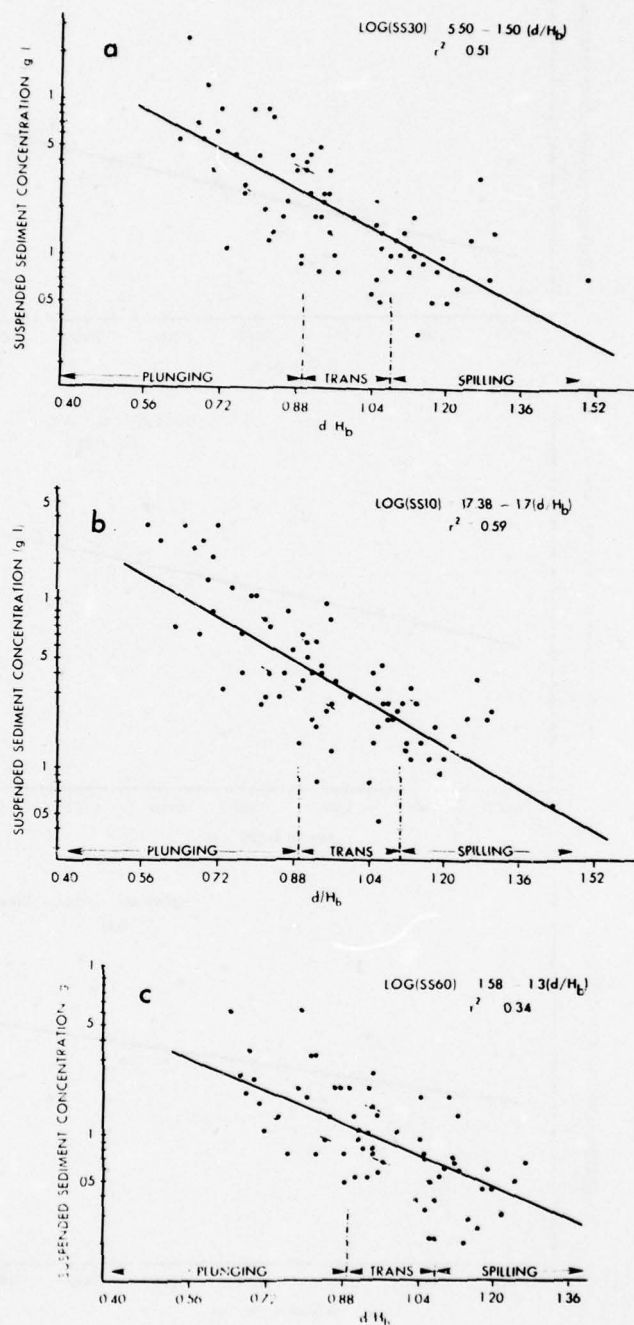


Figure 38. Mean SS10 (a), SS30 (b) and SS60 (c) vs. parameter d_b/H_b showing good separation of concentration values for the indicated breaker type ranges. Up to 60% of the variability in mean concentration is accounted for by this simple variable. The improved correlation over the results using parameter BRKER are partly due to the reduced number of mean values.

unaccounted for by this parameter, it is a continuous variable, more useful for sorting the data and obtaining correlations than a simple stepped visual classification.

In the following results, data sorting has been performed on the basis of the d_b/H_b breaker classification given in the previous section. At the discretion of the writer, a slight modification to the d_b/H_b ranges by wave type were made in order to increase the number of data points for each sample. Sorting by breaker type was performed using the following ranges:

Spilling - $d_b/H_b > 1.04$
 Transition - $0.89 < d_b/H_b < 1.10$
 Plunging - $d_b/H_b < 0.93$.

Thus, there was overlapping of the data with some data classified as both spilling and transition waves ($d_b/H_b = 1.04-1.10$) and some as plunging and transition waves ($d_b/H_b = 0.89-0.93$). This is deemed justified since, by definition, transition waves exhibit characteristics of each primary breaker class, and the indicated ranges maintain the distinction.

Tests of Concentration vs. Wave Process Parameters

It would be useful to establish the relationship between suspended sediment concentration and commonly measured wave parameters for purposes of prediction. The writer is unhappy to report, however, these much sought after simple relationships do not exist for these data. The following results provide some clear trends, but many relationships are, at best, ambiguous and require additional data for testing. Many of the independent variables offer little explanation for the variation in suspended sediment.

Breaker height (H_b). - Sorting the data by d_b/H_b breaker type ranges, SS10 is plotted against H_b in Figure 39. (Note: the relation between SS10, SS30, SS60 and SS100 should be apparent from preceding sections. To conserve space, only SS10 will be used in the following plots).

The data included a range of wave heights from approximately 20 to 150 cm for each breaker type. Each data point represents the mean concentration for a particular wave height (with ± 1 standard deviation indicated).

The clearest trend is, of course, the expected higher concentration values in plunging than in spilling waves. But the variation of concentration by wave height for a given breaker does not appear to follow the widely-held notion of increasing suspensions with increasing wave energy, at least for moderate swell conditions. There is a statistically significant trend of decreasing concentrations with wave height in plunging waves (Fig. 39a). Transition waves appear to attain maximum concentrations at some intermediate wave height; whereas spilling waves show the least variation with height.

It is not clear why this unexpected result occurs, but it could be due to several reasons. Smaller waves are generally of short period, located closer to the swash zone where swash uprush-backrush interactions are greatest, and suspensions are more frequent, according to Brenninkmeyer (1976b). Large waves tend to break in deeper water considerably seaward of the zone of maximum backrush. An attempt was made during the field sampling to selectively pick waves which were least affected by the backrush of the previous wave since the problem of uprush-backrush interactions introduces an additional

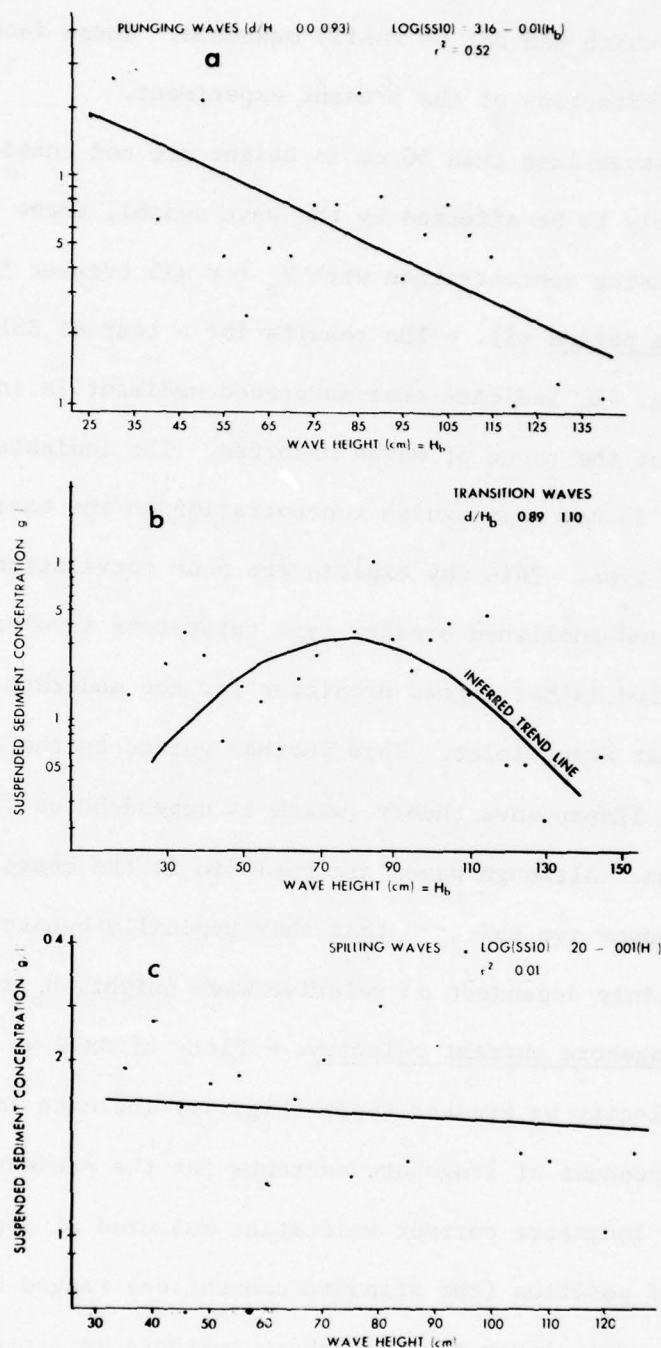


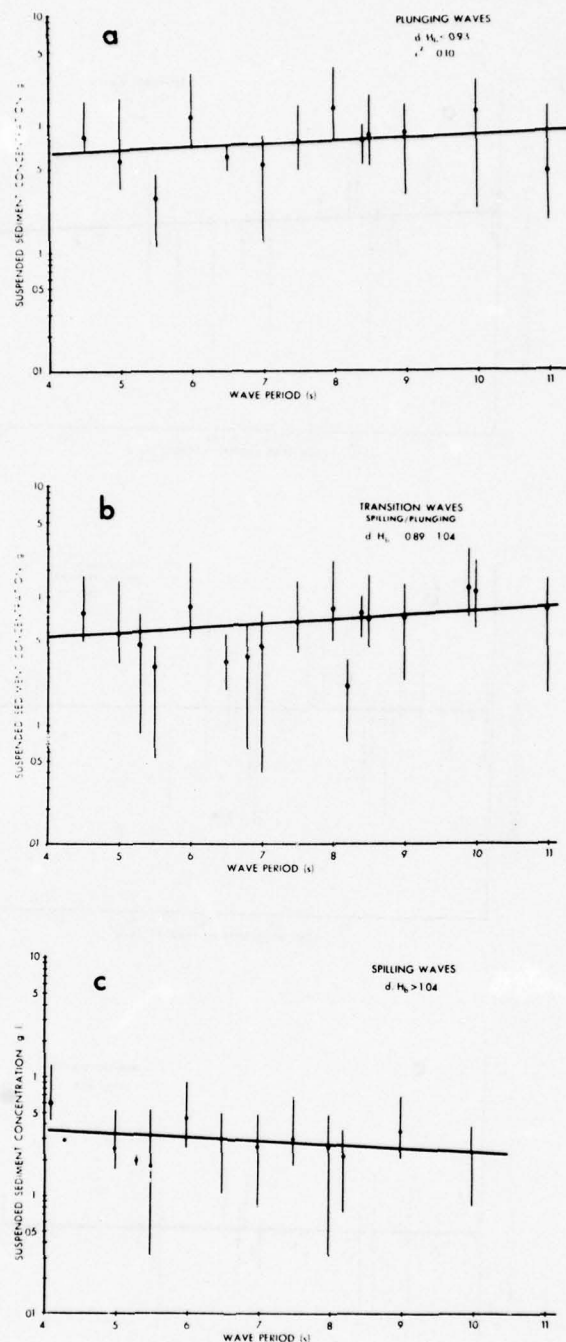
Figure 39. The variation in mean SS10 with wave height for the 3 commonly observed breaker types, plunging, $d_b/H_b > 0.93$ (a), transition (b) and spilling, $d_b/H_b > 1.04$ (c). Despite relatively low correlations, there is significant trend of decreasing concentration with wave height for plunging waves.

variable which can not be easily measured. These factors point to some of the limitations of the present experiment.

If waves less than 50 cm in height are not considered (those most likely to be affected by the wave swash), there remains a trend of decreasing concentration with H_b for all breaker types.

Wave period (T). - The results for a test of SS10 vs. T by breaker type (Fig. 40) indicate that suspended sediment is independent of wave period for the range of waves observed. The indicated trend lines are flat and do not distinguish concentration on the basis of period for any wave type. This may explain the poor correlation between concentration and published breaker type parameters involving wave steepness. Wave period is not a good predictor for the moderate swell conditions sampled at Price Inlet. This further points to the limitations of applying linear wave theory (which is dependent on T) to surf zone phenomena. Although waves are periodic at the coast, Figure 40 gives supportive evidence that they typically behave as solitary waves mainly dependent on relative wave height, H_b/d_b .

Longshore current velocity. - Plots of SS10 vs. longshore current velocity by breaker types (Fig. 41) indicate that concentration is independent of longshore currents for the present range of data. Surface longshore current velocities measured at approximately the mid-surf position (the standard convention) ranged up to 70 cm/s, significantly lower than longshore currents in storms on the South Carolina coast (Finley, 1976). While it has been shown that less force is required to suspend a particle rolling along the bed (Bag-nold, 1947), this effect does not appear to be an important factor causing an increase in concentration with velocity for these data.



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Figure 40. Mean SS10 vs. wave period for plunging (a), transition (b) and spilling (c) breakers, showing virtually no dependency between the two variables for these data collected during moderate swell conditions. Bars on the data points represent the range of concentration values to ± 1 standard deviation.

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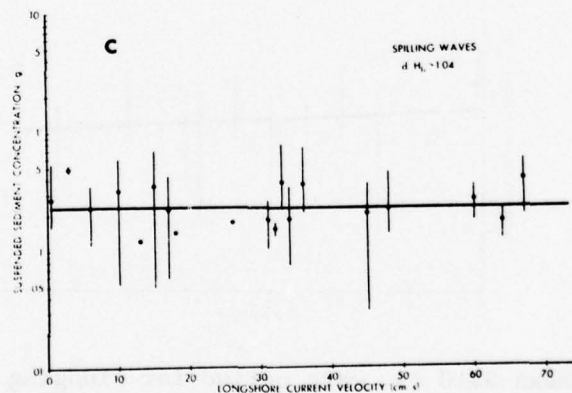
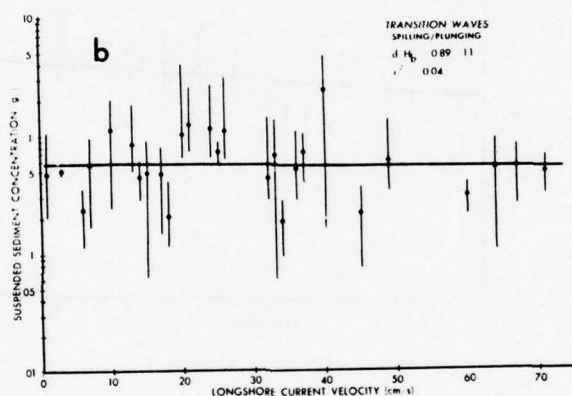
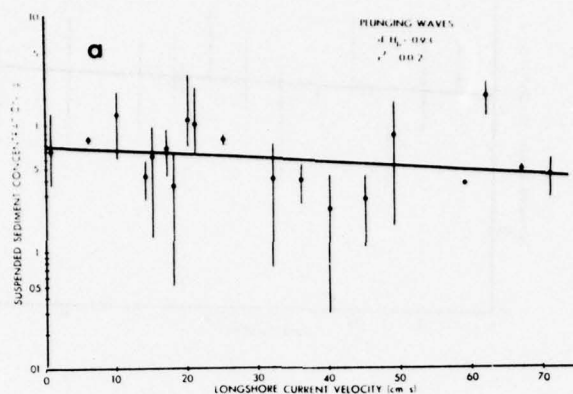


Figure 41. Mean SS10 vs. longshore current velocity for plunging (a), transition (b) and spilling (c) breakers, showing total lack of dependency between the two variables. Correlation coefficients are less than .05 in all cases.

Distance from breakpoint. - The distribution of suspended sediment with respect to distance from the breakpoint for each wave type is given in Figure 42. The data appear to fit the expected trend of maximum concentration within a few meters of the breakpoint. In general, sediment suspension remains at a low level of the same order of magnitude at, or seaward of, the breaker line. There is a rapid increase in concentration in the landward direction near the breakpoint, then a slow decrease toward shore. These data do not include measurements in the lower swash zone, where, according to Breninkmeyer (1976b), sediment suspension reaches a maximum.

One interesting trend in these data is the peaked shape of the inferred distribution curve for plunging waves. Suspended sediment is higher in plunging than in spilling waves, but reaches maximum concentration values over a narrower portion of the surf zone. Spilling waves appear to attain a particular concentration level which is more or less maintained across the surf zone. This offers another line of evidence supporting the notion of variable rates of energy dissipation in different breaker types and agrees with the work of Führböter (1970).

Other process variables. - Similar tests of suspended sediment dependency were performed on wind velocity, tide elevation, breaker angle, and so on, but none were found to be significantly correlative (see correlation coefficients in Tables 7 and 8 in a later section).

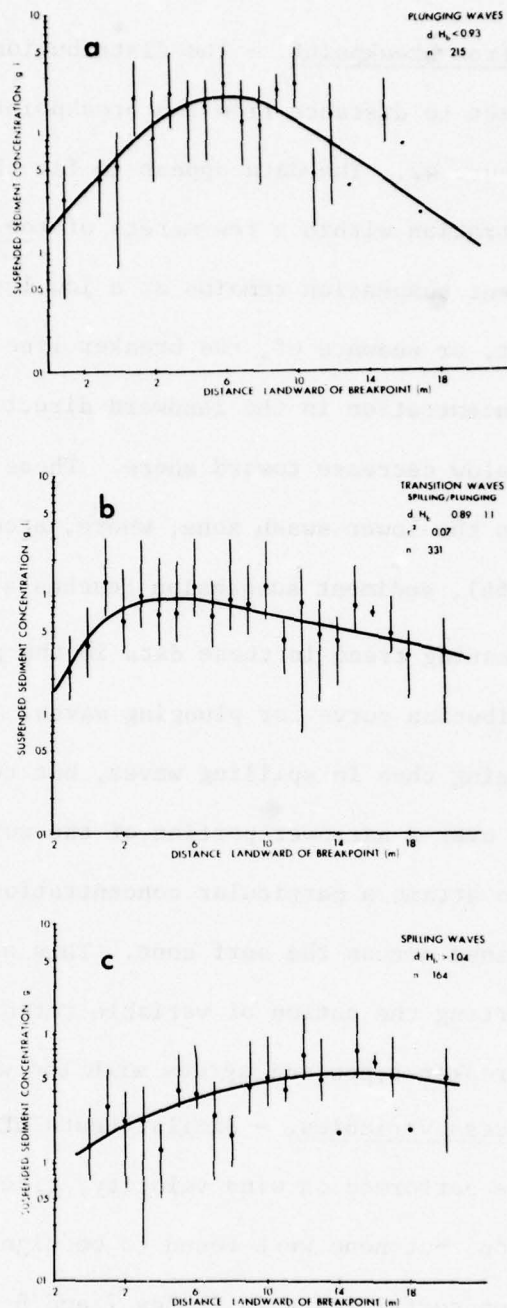


Figure 42. Inferred variation in SS10 vs. distance from the breakpoint for plunging (a), transition (b) and spilling (c) waves. In plunging and transition waves, there is a rapid increase in concentration within a few meters landward of the breaker line, then a gradual decrease in the shoreward direction. Spilling waves appear to reach a maximum concentration approximately 10 m landward of the breakpoint which is more or less maintained across much of the surf zone.

Size Distribution of Suspended Sediment

Relatively little data exists regarding the size distribution of suspended sediment in the surf zone. Fairchild (1977) has performed more size analyses on samples of suspended sediment than any other investigator and confirms the notion of decreasing grain size from bed to suspended samples from Ventnor, New Jersey and Nags Head, North Carolina (Table 4). The data of Figure 43 from Fairchild show an interesting trend of constant median grain size with nozzle (pump intake) elevation, but a general increase in size of the coarsest fraction near the bed. This reflects not only the intermittent nature of the suspension, but reduced competency with elevation above the bed.

Selected samples were sized in the present study in order to determine if there were significant variations predictable by the independent wave parameters and which, perhaps, influence the total concentration in suspension. The data presented are based on suspended sediment samples collected during the principal field study in 1977 as well as several earlier experiments in fall 1975. Table 5 gives an overall summary of grain size statistics by date, station, and elevation above the bed. Included are corresponding bottom sample sizes for the 1975 data and mean concentration values for each sample position.

Size distribution with respect to sample elevation. - Table 5 shows the general trend of decreasing grain size with elevation above the bed and lower sizes among suspended sediment samples compared with bed samples. Size distributions are typically coarse skewed. Figure 44 is a representative set of frequency and cumulative frequency size

Table 4. Size difference between suspended and bottom samples, obtained from a tractor mounted pump sampler (After Fairchild, 1977; p. 28).

Locality	Date	Station	Median sand size (mm)	
			Bottom	Suspended
Nags Head, N. C.	Apr. 1964	320	0.30	.
		285		0.22
		350	0.35	
		352		0.16
		765	0.23	
		758		0.16
Ventnor, N.J.	May 1965	Beach	0.20	
		Typical		0.13
Ventnor, N.J.	Mar. 1971	360	0.22	
		370		0.18
		375	0.22	
		375		0.19
		385	0.22	
		388		0.18

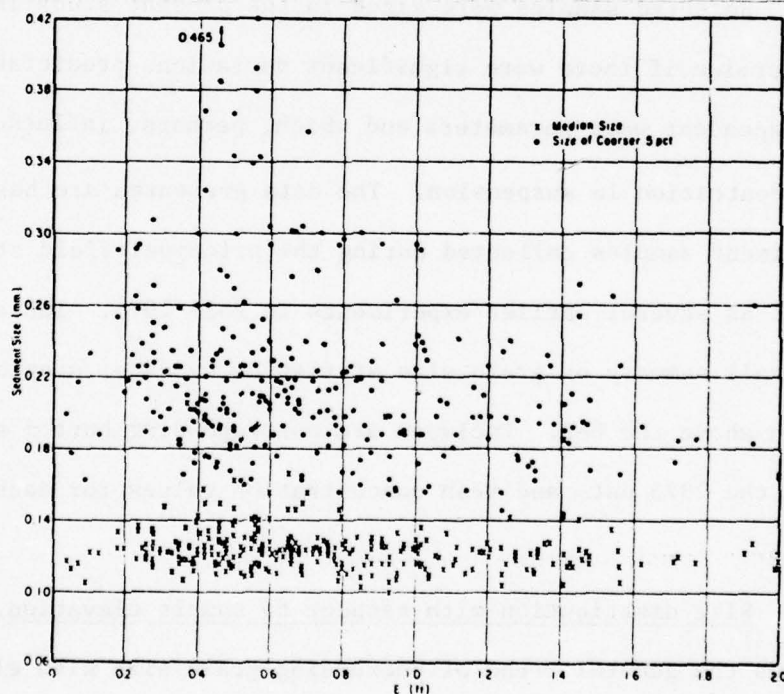


Figure 43. Median (x's) and coarse (dots) suspended sediment sizes by nozzle elevation above the bed (E) for time-averaged pump samples. Note general increase in size of the coarsest fraction near the bed. (From Fairchild, 1977; Fig. 11).

Table 5. Grain size statistics-suspended sediment and corresponding bed samples, by date, station and sample position.

Overall	1975						Mean con. (g/l)+	n
	$m_z^*(mm)$	$m_z^*(\phi)$	$\sigma(\phi)$	skew	kurt			
Suspended samples	.135	2.893	.480	-.345	2.734	1.53	66	
Bed samples	.153	2.735	.550	-.281	2.301	----	28	
Station BU2								
SS60	.110	3.205	.514	-.783	6.118	.70	4	
SS30	.099	3.337	.327	-.072	1.029	.85	7	
SS10	.130	2.997	.448	-.506	3.335	3.61	11	
Bed	.161	2.683	.638	-.207	2.211	----	13	
Station CA1								
SS100	.145	2.780	.486	-.365	1.067	.53	1	
SS60	.130	2.961	.417	-.325	2.265	.74	10	
SS30	.121	3.057	.440	-.645	6.606	.91	13	
SS10	.130	2.963	.438	-.254	1.234	3.40	20	
Bed	.146	2.780	.473	-.426	2.379	----	15	
Overall	1977						Mean con. (g/l)+	n
	$m_z^*(mm)$	$m_z^*(\phi)$	$\sigma(\phi)$	skew	kurt			
Suspended samples	.110	3.190	.551	-.612	3.172	1.12	129	
Station BU2								
SS60	.090	3.47	.257	-.285	-----	.55	1	
SS30	.100	3.32	.524	-.630	4.037	.74	27	
SS10	.109	3.20	.603	-.693	3.480	1.26	37	
Bed		- not sampled -						
Station CA1								
SS60	.101	3.305	.435	-.800	5.601	.61	2	
SS30	.111	3.166	.540	-.561	2.908	.76	25	
SS10	.115	3.121	.536	-.517	2.110	1.47	35	
Bed		- not sampled -						

* M_z = mean diameter

+These concentrations are higher than overall means for all suspended sediment concentrations because only samples with high concentrations have adequate masses for sizing using the HESA.

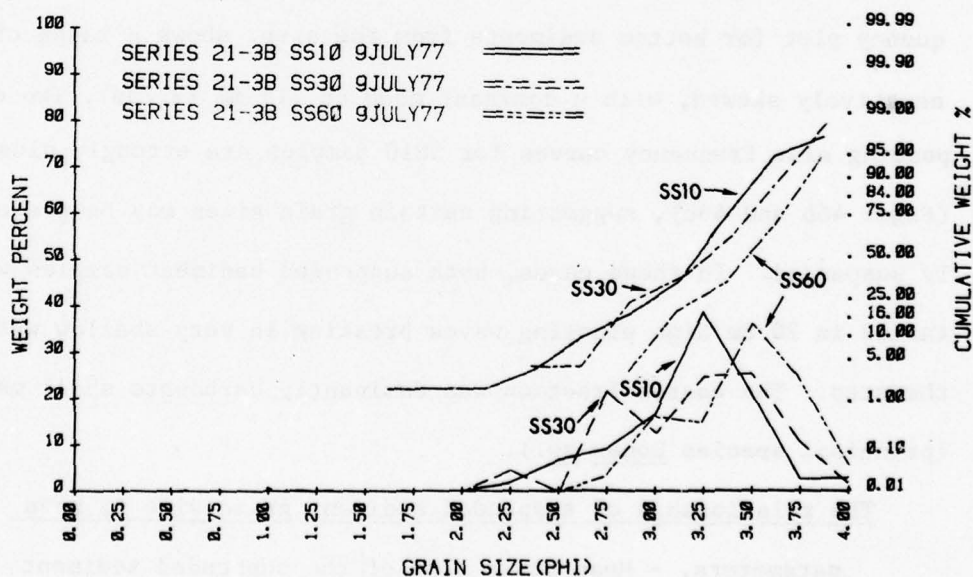


Figure 44. Example frequency and cumulation frequency size distribution curves for suspended sediment samples collected at 10, 30 and 60 cm above the bed in a single vertical array. Note general decrease in mean size with elevation. Size distributions determined from settling velocities using HESA (Anan, 1972).

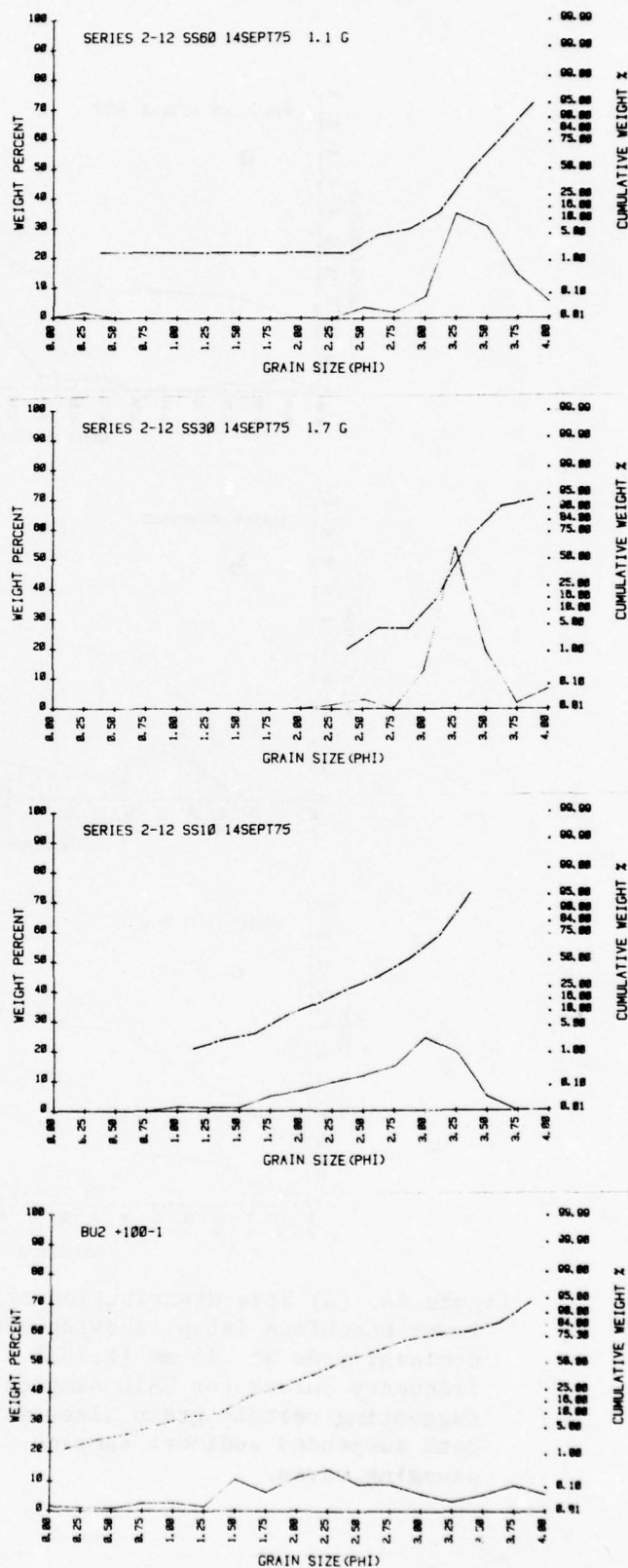
distribution curves for one array of suspended sediment samples. The superimposed curves show the typical decrease in mean grain size from the sample near the bed (SS10) to the uppermost sample (SS60). A sequence of size curves in Figure 45 includes the corresponding bed sample (Fig. 45d). Note, in this particular case, the wide range of sizes on the bed, but decreasing range of sizes in suspension going up in the water column. Although this bed sample is not typical of the average size distribution along profile BU2, the widest range of grain sizes often occur at the lower beach face (step) or seaward side of the outer ridge. The location "+100-1" labeled on Figure 45d is an example from this latter location. Figure 17 shows the distribution of grain sizes at station BU2 on 20 August 1975, with the corresponding beach profile in Figure 17g.

The data also show some interesting reversals in the normal trend of negatively skewed distribution. Figure 46a, containing a size frequency plot for bottom sediments from the step, shows a range of sizes, negatively skewed, with a dominant mode at .15 mm (2.75 ϕ). Two corresponding size frequency curves for SS10 samples are strongly plus skewed (Figs. 46b and 46c), suggesting certain grain sizes may be preferentially suspended. In these cases, both suspended sediment samples were obtained in 20 cm high plunging waves breaking in very shallow water at the step. The coarse fraction was dominantly carbonate shell material (principal species Donax sp.).

The relationship of suspended sediment grain size to wave

parameters. - Mean grain size of the suspended sediment samples were sorted by wave height and breaker type to determine if

Figure 45. Grain size frequency curves for an array of suspended sediment samples and corresponding bed sample. From top to bottom, size distribution for SS60, SS30, SS10 and the bed. Note wide range of sizes on the bed at this location. The label +100-1 in (d) refers to distance in meters from a benchmark on land. The range of sizes and mean size decreases for suspended samples going up in the water column.



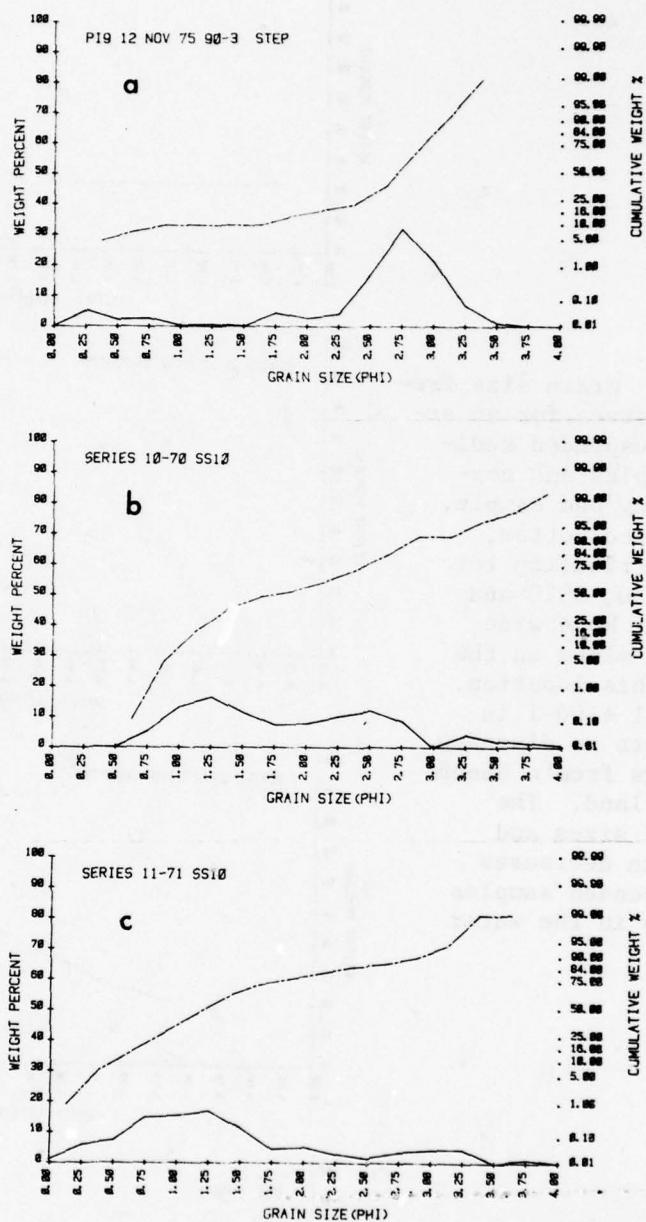


Figure 46. (a) Size distribution of sediment from the bed at the lower beachface (step) showing common negative skewness with a dominant mode at .15 mm (2.75 ϕ). (b) and (c) Corresponding size frequency curves for SS10 samples which are strongly plus skewed, suggesting certain grain sizes may be preferentially suspended. Both suspended sediment samples were collected in 20 cm high plunging waves.

Table 6. Grain Size Statistics (1977) - Suspended Sediment Samples by Breaker Type and Sample Position.

Sample elevation	M_z (mm)	SPILLING			Skew	Kurt	n
		M_z (ϕ)	σ (ϕ)				
SS60	.090	3.470	.251	-.285	-0.43	1	
SS30	.110	3.178	.551	-.698	2.53	4	
SS10	.109	3.202	.560	-.779	4.29	10	
Sample elevation	M_z (mm)	PLUNGING			Skew	Kurt	n
		M_z (ϕ)	σ (ϕ)				
SS60	.101	3.305	.435	-.800	5.601	2	
SS30	.105	3.258	.510	-.584	4.228	35	
SS10	.113	3.147	.570	-.634	3.006	52	

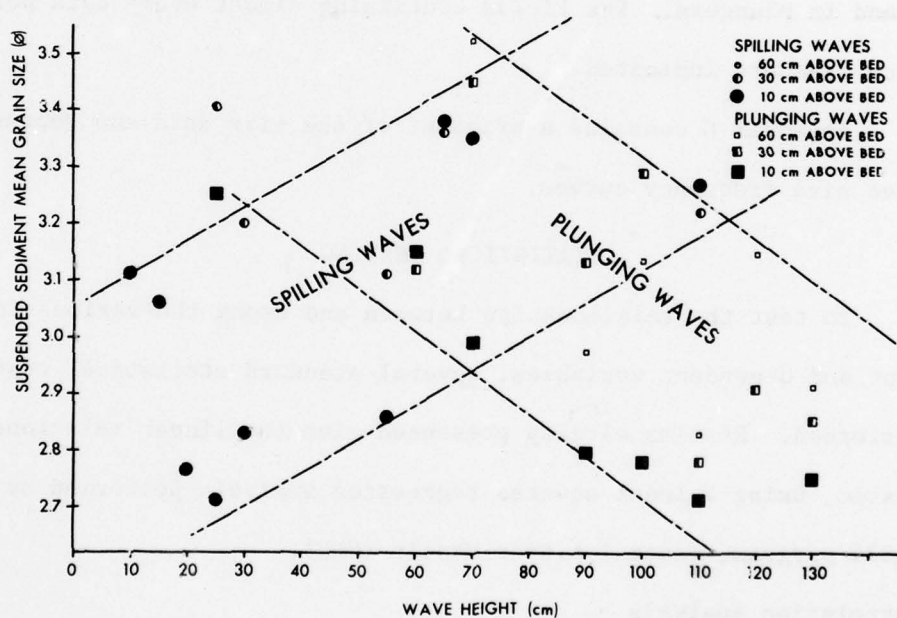


Figure 47. Mean size of suspended sediment averaged by wave height and sorted by breaker type and sample elevation. Circles represent spilling waves, and squares correspond to plungers. Symbol size decreases with sample elevation above the bed. For a given wave height, there is a consistent trend of decreasing mean size with elevation. The fields for plunging and spilling breakers containing virtually every data point are indicated. It is uncertain whether this is real or an artifact of the sampling method.

there was any dependency. The overall mean sizes by breaker type given in Table 6 do not indicate any obvious trends other than uniformity at a given sample position.

Figure 47 is a scatter plot showing mean grain sizes by sample height, wave height, and breaker type. Squares correspond to plunging waves and circles to spillers. At a given wave height, a vertical sequence of different sized squares or circles correspond to each sample elevation with the largest data points representing the SS10 samples and so on. There is an apparent trend of decreasing size with increasing wave height in spilling breakers and the reverse trend in plungers. The fields containing almost every data point for each case are indicated.

Appendix C contains a printout of the size data and representative size frequency curves.

STATISTICAL TESTING

To test the relationships between and among the various independent and dependent variables, several standard statistical tests were performed. Results already presented give the linear relationships tested, using a least squares regression analysis performed by the SAS76 program, General Linear Models (GLM).

Correlation Analysis

Correlation matrices were produced using SAS76 Procedure CORR for the principal data set of concentration values, as well as the data set of size distributions. A partial listing of each correlation matrix is given in Tables 7 and 8. The complete matrix for 408 observations is given in Appendix D. With regard to the log transformations of the dependent variables, SS10, SS30 and SS60, correlations

were highest:

- 1) between suspended sediment samples at different elevations (e.g. for Log (SS10) with Log (SS30), $r = .809$);
- 2) between concentration and breaker type parameters, BRKER and d_b/H_b ; $r = -.544$ and $-.535$, respectively; and
- 3) between concentration and breaker depth (d_b) or concentration and beach slope (m); $r = -.334$ and $.261$, respectively.

Correlation coefficients between suspended sediment concentration and the independent variables wave period, longshore current velocity, and wind velocity, were all positive, but less than 0.2. Galvin's (1968) breaker parameter (B_b) shows slightly negative correlation with concentration ($r = -.15$). With no separation of the data by breaker type, wave height shows almost no correlation with Log (SS10) ($r < 0.01$).

The independent variables are somewhat more correlative with concentration when the data is sorted by " d_b/H_b " breaker types. For example, the correlation with wave height for plunging waves improves to $r = .217$, and with beach slope to $r = .243$.

Given the inherent variability of the data, these low coefficients are not surprising. But are they significant, or is sediment suspension in the surf zone basically a random process? Calculated against individual observations, correlations are certainly poor. But when calculated against mean concentrations obtained under similar conditions, they improve significantly.² A few examples of this are given in Table 9.

² The means referred to are those plotted in the scatter diagrams in the preceding sections.

Table 9. Improved Correlation Coefficients using Mean Suspended Sediment Concentrations.

<u>Log (SS10)</u>	with		<u>r</u> (means)	<u>n</u>	<u>r</u> (obs)
		d_b/H_b	.77	72	-.53
""		BRKER	.64	165	-.41
""		m	.56	31	.26
""		H_b - spilling	.10	17	-.07
""		H_b - plunging	.72	21	-.52
""		T - plunging	.10	18	.07
""		V - transition	-.06	27	.02

Although there is little significant change in correlation between wave process parameters and concentration, with the exception of H_b in plunging waves, correlations with the breaker type parameters and beach slope are considerably improved.

Multiple Regression

The SAS76 Procedure STEPWISE was used to perform multiple regression analysis on the principal variables. Details of the method of computation are given in Barr, et al., (1976). The dependent variables tested were Log (SS10), Log (SS30) and Log (SS60). Independent variables included the breaker parameters, wave process parameters, slope and sample positioning variables. The point of the analysis is to determine which variables should most likely be included in a regression model.

The simplest form of regression analysis is the linear model between a dependent and one independent variable (the linear regression

calculated for most of the scatter plots). The linear model plots as a line in space defined by the dependent variable on one axis (usually y-axis) and an independent variable on another axis (x-axis). As additional variables are added, the model assumes more dimensions. A 2-way model, with 2 independent variables, plots as a plane defined by the y, x and z axes. The number of variables determines the number of dimensions in space needed to define the model. It is generally desirable to add variables to the model as long as there is continued significant improvement in the fit of the data. This, of course, is indicated by the correlation coefficient squared (r^2), which is a measure of the percentage of variation in the data accounted for by the model.

There is no guarantee that models such as these represent real processes precisely, but they can be valuable in isolating the most important variables.

Selected results using procedure STEPWISE are given in Table 10 in terms of the best 2-way, 3-way and 4-way models calculated. As shown in the table, one independent variable (the 1st entered in each model) generally accounts for most of the variation. Succeeding variables only give slight improvement in r^2 . For individual observations, less than 40% of the variation is explained by any of these models. However, when mean values of concentration (by d_b/H_b) are used (Table 10 - lower half), the model improves significantly.

There is a relatively small improvement in r^2 with increasing number of variables indicating the linear, or 2-way, models are probably the most reliable. And, not surprisingly, the key variables are the breaker type parameters, d_b/H_b and BRKER.

Table 10. Multiple regression models on dependent variable Log (SS10) (intercepts and slopes omitted)

INDIVIDUAL OBSERVATIONS OF LOG(SS10)

ALL WAVES

Model	r^2
BRKER; d_b	.378
BRKER; d_b ; Distance from breakpoint (x)	.394
BRKER; d_b ; X; V	.400

PLUNGING WAVES

d_b ; BRKER	.370
d_b ; BRKER; X	.384
H_b ; d_b ; X; m	.393

SPILLING WAVES

H_b ; d_b/H_b	.044
H_b ; d_b/H_b ; V	.084
H_b ; d_b/H_b ; T; X	.094

MEAN VALUES OF LOG(SS10)

ALL WAVES

Model	
d_b/H_b ; d_b	.641
d_b/H_b ; d_b ; m	.652
d_b/H_b ; H_b ; d_b ; m	.662

DISCUSSION

Limitations of the Data

The data presented are limited due to the relatively restricted site specific conditions of the experiment. Although there has been considerable mention of varying beach slopes and breaker type, the data is confined to gently sloping, fine-grained, densely compacted beaches. As such, it does not represent a wide spectrum of coastal morphologies or sediment grain sizes. With regard to the three underlined conditions, all are important in influencing the amount of sediment in suspension.

For example, despite some ambiguity in these data, it appears that even in a narrow range of beach slopes, concentration tends to increase with increasing slope. Data from steeper beaches at Duck, North Carolina ($\bar{m} = .05$) collected using the same techniques, yield higher mean concentrations near the breaker line (Kana and Ward, in prep.). Nummedal and Stephen (1976) measured suspended sediment concentrations two orders of magnitude higher than the Price Inlet data on Alaska beaches ($\bar{m} = .074$). Despite significant variations in wave climate, among these examples, beach slope should be considered in any general predictive model.

Two factors affect compaction of the bed: sediment grain size distribution and wave energy. Since most beaches are generally well sorted, with an absence of cohesive sediments, finer-grained beaches tend to be most densely compacted. Beaches of a given grain size show variations in bed compaction with respect to beach morphology and exposure to waves. For instance, the exposed beach face and seaward side of the inner ridge at the present study site is a smooth, compact pavement, dense enough to pull a 250 kg cart along without leaving deep tire tracks (Fig. 23). But

near the inlet, where the beach is more sheltered from waves by offshore shoals, the sand is sometimes thixotropic. Obviously, these two conditions affect sediment suspension quite differently. There was no attempt in this experiment to determine the magnitude of these differences.

The data only apply to relatively featureless surf zones, essentially devoid of small scale bedforms. Although there have been several studies on the effect of bedforms on sediment suspension and transport, they have relatively little application for this experiment. Along exposed beaches in South Carolina, small-scale ripples are essentially absent from the breaker zone and were not treated as a significant variable in these data.

A final consideration which necessarily limits these data is the zone of sampling. As mentioned previously, an attempt was made to include only data collected near the breaker zone away from the swash zone.

Advantages of the Data

Waves were chosen with the intent of sampling only in well formed, easily definable breakers. This certainly biases the data in favor of swell conditions, but increases control over sampling position and wave measurements.

Other advantages of these data compared with previous, direct measurements of suspended sediment include:

- 1) Multiple samples to determine vertical gradients of concentration;
- 2) Multiple arrays to "follow" the variation in concentration in the bore of a broken wave.

3) Minimal sampler influence to the bed and relatively fast response time; and

4) Wave process measurements, of necessity, made in situ.

This last advantage gives an unusual amount of control to these data, compared with experiments utilizing remote sensing equipment. In the case of breaker measurements, remote sensing wave gauges are inadequate for determining the breakpoint or distance to the breakpoint from the sampling station. Similarly, depth at breaking at the breakpoint can only be reliably measured by an observer in situ.

The final advantage is the relatively large number of suspended sediment samples collected under similar site conditions. This is a necessity to distinguish any real trends.

Controlling Factors of Sediment Suspension

The results contained herein tend to indicate that suspended sediment in the surf zone occurs in a somewhat predictable manner given information on certain wave process variables. The data tend to confirm certain notions including the dependence of concentration on elevation above the bed and breaker type. But at the same time, they present relations which are not easily explained based on our present understanding, as, for example, an apparent decrease in concentration with wave height in plunging waves.

The range of suspended sediment concentrations found in the surf zone is at least several orders of magnitude, ranging from a few milligrams per liter to several tens of grams per liter. In terms of the distribution of concentration, the greatest fluctuation is in a vertical column. Suspensions of sand are intermittent; consequently, the variation is dependent on the elevation to which these bursts are thrown. From a statistical standpoint, the highest frequency of bursts from the bed will occur near the bed, making the mean concentration highest close to the bottom. These data show a previously observed trend of exponential decay of concentration in the vertical. Above 60 cm, sand suspensions are rare and at higher elevations, concentration tends toward some uniform wash load value composed entirely of fine-grained particles. Variability is greatest near the bed. Elevation, then, causes a natural separation of mean concentration values and can be used to sort suspended sediment samples for comparison with different sites.

For a given elevation, the most important controlling factor, based on these data, is breaker type. Plunging waves typically suspend an order more sediment than spilling waves. The relatively low concentrations characteristic of spillers suggests that relatively little sand

moves by suspension; whereas plunging waves commonly exhibit bursts of coarse sediment "boiling" to the surface. This has several important implications to longshore and onshore/offshore transport which will be discussed in the last section. A partial explanation for this was given in Figure 7 (Miller, 1976), showing the distribution of air bubble concentration in spilling and plunging waves.

Figure 48, from Iverson (1952), which shows the velocity fields in spilling and plunging waves, offers more evidence for the variation in concentration by breaker type. Note in the trough preceding the crest of a plunging wave, the velocity component parallel to the bed is up to 10 times higher than in a spilling wave. This causes initial motion of the grains in a seaward direction. As the wave meets the grains, there is a sudden reversal of the current first upward toward the crest, then landward. Thus, a particle in a plunging breaker will typically describe a C-shaped trajectory as it is placed in suspension. Spilling waves, with much lower velocities in the trough, do not produce the same degree of seaward rolling along the bed. As the crest passes in a spiller, there is relatively little vertical velocity with most of the flow moving landward, parallel to the bed. The effect of this is to "push" sediment landward a short distance in brief impulses.

The other fundamental difference between the two breakers is the ratio d_b/H_b , and beach slope, m , shown in Figure 48. For waves of a given H_b , a smaller "cushion" of water exists under the plunging wave allowing proportionately more energy to reach the bed and a higher frequency of suspensions to occur.

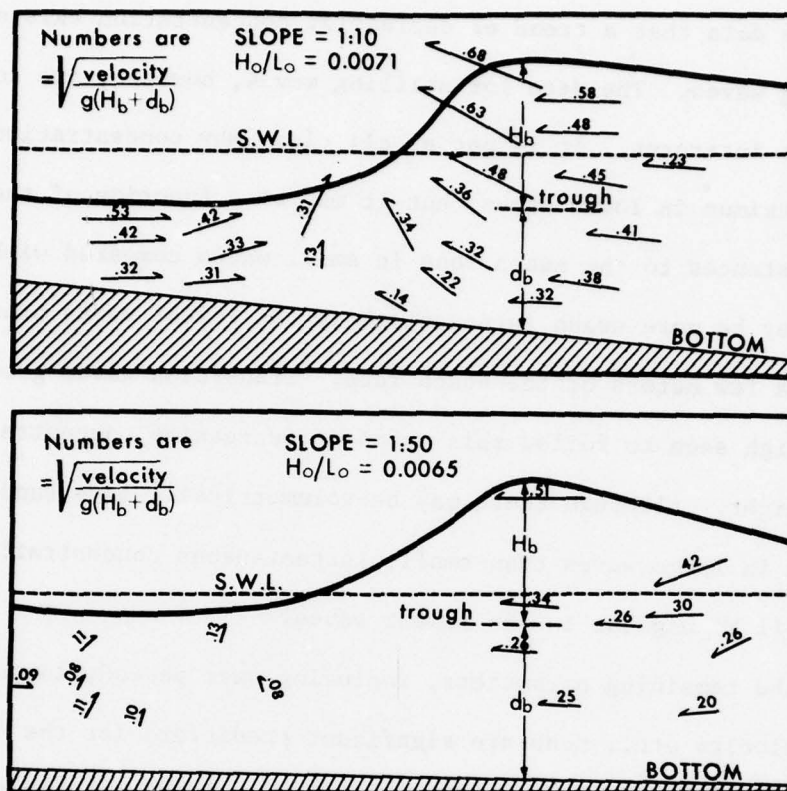


Figure 48. Relative distribution of velocity vectors in plunging (upper) and spilling (lower) breakers. Units are in terms of $(V/g(H_b + d_b))^{1/2}$. Note the velocity field is much stronger in plunging waves causing more rolling of sediment along the bed in both the onshore and offshore direction (Modified after Iversen, 1952).

From the results and the foregoing discussion of velocity fields in the primary breaker types, it is apparent that distance from the breakpoint is a controlling factor on the observed sediment concentrations. This seems more the case in plunging than spilling waves, based on Fig. 42a, which shows maximum concentrations peaking close to the breakpoint. Spilling waves appear to cause a slower increase in concentration in the landward direction and maintain that level across a wider portion of the surf zone. Seaward of the breaker zone, suspended sediment decreases rapidly.

With regard to increasing wave height, it's possible to conclude from the data that a trend of decreasing concentration exists for plunging waves. The data for spilling waves, however, are too scattered to interpret. It is not at all clear why concentration should be at maximum in lower waves, but it may be a function of the relative distances to the swash zone in small waves compared with large. There may be more swash interactions in the lowest waves breaking within a few meters of the beach face. Transition waves greater than 50 cm high seem to follow this trend of decreasing concentration with wave height. Although there may be volumetrically more sand in suspension in large waves than small, instantaneous concentrations may very well be highest in the lowest waves.

Of the remaining parameters, including wave period, longshore current velocity etc., none are significant predictors for the present data. This may be due to the limited range of values obtained for most of these parameters while sampling under moderate swell conditions. Possibly, given a wider range of conditions, some additional variability would occur. But, it is unlikely that any of these remaining process variables overshadow the effect of the primary controlling factors, even under extreme conditions.

Figures 49a-e summarize what are considered to be representative general models for the distribution of concentration in the breaker zone. While these relationships can only explain up to 65% of the variability in the present data, they suggest that suspended sediment is somewhat predictable for particular site specific conditions. The parameter, d_b/H_b , has been shown to reasonably sort breaker types and accounts for most of the variability at a given sample elevation.

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Among these models, the one with the most uncertainty is concentration's dependency on wave height. More data are required to test whether the model proposed herein is truly applicable.

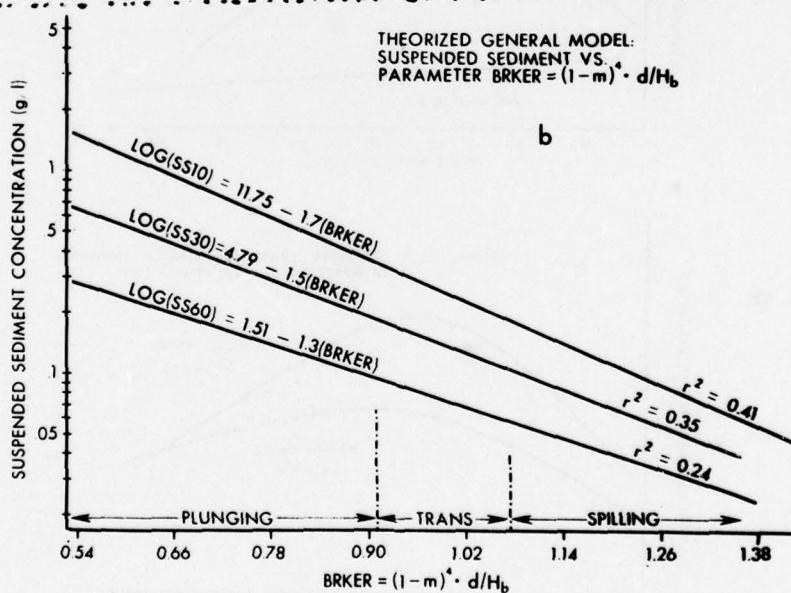
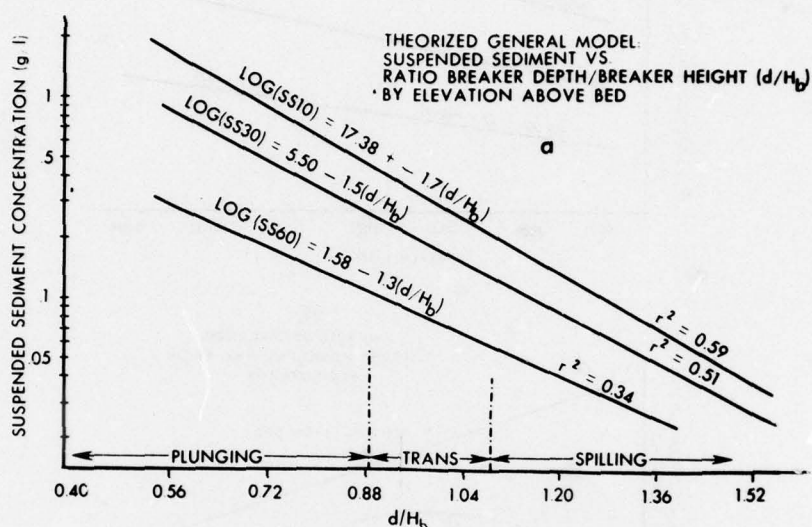


Figure 49(a and b). General linear models for the distribution of suspended sediment concentration in the breaker zone, based on the present experiment. (a) Ratio d/H_b by sample position and (b) Parameter BRKER by sample position.

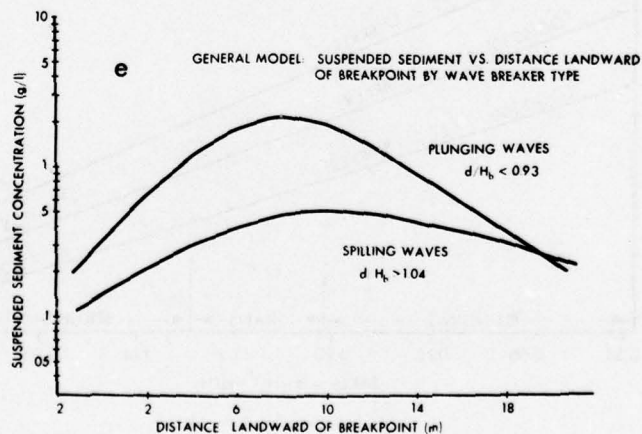
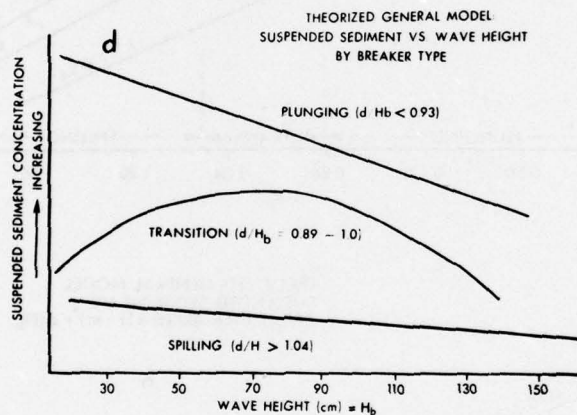
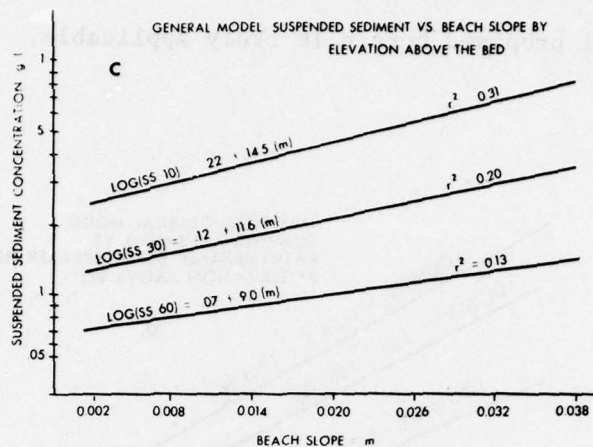


Figure 49(c, d and e). General models for the distribution of suspended sediment concentration based on: (c) beach slope (m) by sample position; (d) wave height by breaker type and (e) distance from the breakpoint by wave type.

Implication of Results

Equilibrium profiles. - These data contain some possibly significant applications to onshore/offshore sediment transport and equilibrium profiles due to the large difference in sediment suspension between spilling and plunging waves. For instance, there exists a model by Dean (1973) in which the onshore/offshore movement in the surf zone is determined by the relative height of a suspension and fall velocity of the sediment with respect to wave period. Briefly, Dean's model considers that sand is placed in suspension during wave breaking to some level, (S), which is proportional to H_b :

$$S = \beta H_b \quad (6)$$

Proportionality coefficient, β , is less than one, but of the same order. The sediment in suspension has a settling velocity, W_D , which can be combined with S to determine the time, t, required for the particle to settle to the bed, i.e.,

$$t = \frac{S}{W_D} \quad (7)$$

Assuming asymmetry of water particle motion over the wave period, sediment in suspension will follow two possible trajectories depending on whether t is less than or greater than $T/2$, where T is wave period. As shown in Figure 50 below from Dean (1973; Fig. 2-b), a fall time of less than $T/2$ results in a net onshore motion of the particle; whereas a time greater than $T/2$ produces a net offshore motion.

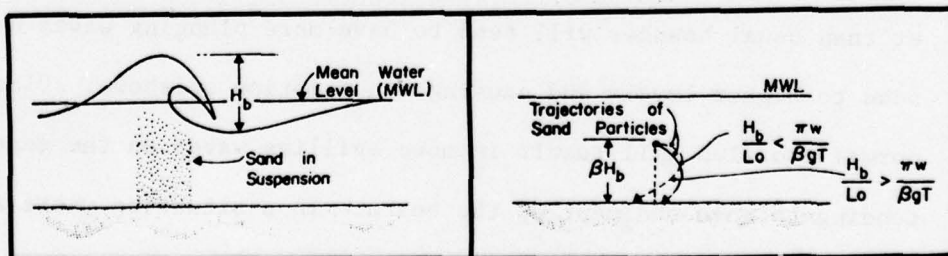


Figure 50. (left) Sediment in suspension at the time of wave breaking. (right) Two theoretical trajectories of a falling particle during the wave period (From Dean, 1973; Fig. 2).

Dean's model formulates this relationship in terms of H_b and T to predict the net movement on the basis of wave steepness. Through linear theory, he derives the following simple relations, using the above criteria to give

$$\frac{H_b}{L_o} < \frac{\pi W_D}{\beta g t} \quad , \text{ onshore movement} \quad (8)$$

$$\frac{H_b}{L_o} > \frac{\pi W_D}{\beta g t} \quad , \text{ offshore movement}$$

where g is acceleration of gravity.

The main problem with this model is the suspension level (S) is proportional to H_b . From the present data, it is evident that d_b/H_b , not H_b alone, controls the suspension level. Instead, S should be predicted from breaker type (ie. d_b/H_b). With this criteria, plunging waves over a range of periods will suspend sediment to higher levels and likely cause net offshore movement; whereas spilling waves over a similar range of periods would suspend sand to lower levels and tend to produce net onshore movement. This can only be described qualitatively, since the velocity field is not symmetric in the surf zone.

With respect to maintenance of the equilibrium profile on beaches, onshore/offshore transport will be a function of breaker type. Steeper than usual beaches will tend to have more plunging waves entraining sand to higher levels and causing a net motion offshore. Flatter than normal profiles will result in more spilling waves on the gentle slope, tending to move sediment up the beach. In a situation where the profile is wavy, breaker type will vary from a higher portion of plungers at the steep parts (causing scour and movement in the offshore direc-

tion) to a predominance of spilling waves along the flats. As the tide rises and falls along this undulatory surface, the breaker type will undergo subtle changes in response to changing slope. The net effect will be a tendency to bring the profile back to equilibrium as sketched in Figure 51. This effect is very common along the South Carolina coast where profile changes are subtle, and the tide range (approximately 2 m) allows waves to break along a wide portion of the profile during every tidal cycle.

Applicability of solitary theory to surf problems. - It has been suggested that linear theory, in particular wave steepness, is not very useful for describing the variability in concentration or breaker type for the present data. Many long regarded notions of sediment transport are based on gross assumptions of idealized sinusoidal waves. For example, the prediction of net onshore/offshore transport from wave steepness, mentioned previously, is based on linear theory, but there is disagreement on the critical steepness value (e.g. Johnson, 1949 ($H_0/L_0=0.025$); King and Williams, 1949 ($H_0/L_0=.012$)).

The waves sampled in the present experiment are best described as solitary waves, independent of period, at least for purposes of determining sediment suspension.

Importance of suspended sediment to total longshore transport. - There is some debate regarding the relative importance of suspended vs. bedload transport in the surf zone, enlivened by a recent paper by Komar (1978). The purpose here is not to debate the issue, but to offer some generalizations based on the present data. Since there is a significant difference in suspended sediment between the two principal breaker types sampled, it is appropriate to consider

them separately.

Spilling waves suspend relatively little sediment, so it would be logical to assume most of the transport occurs near the bed. Plunging waves often entrain moderately high concentrations caused by numerous intermittent suspensions, so the suspension mode may dominate. But to properly evaluate the relative importance of each mode by breaker type, more has to be learned concerning relative rates of transport between these two primary breaker types.

In the opinion of this writer, the major shortcoming of models of longshore transport from wave energy flux is the lack of dependency on breaker type. There has to be a significant difference in the amount of sand transported if there exists large variations in quantities suspended with breaker type. This obviously presents more difficulties in modeling longshore transport, but it is deemed critical to improving our understanding of the relationship between wave energy and sand transport on beaches.

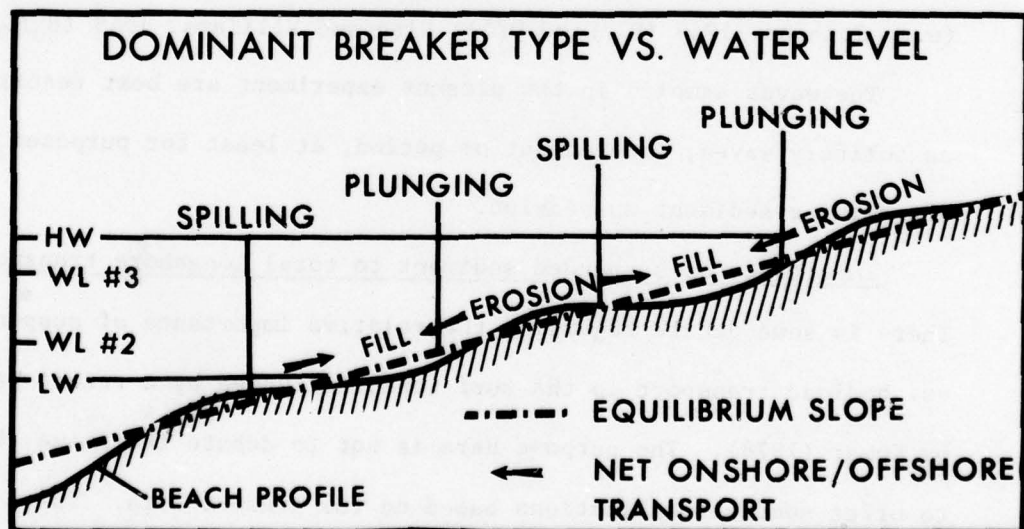


Figure 51. Hypothesized effect of breaker type variation with water level on an undulatory beach profile causing onshore transport when waves are spilling and offshore transport when waves are plunging. The net effect is to return the profile toward an equilibrium slope.

CONCLUSIONS

The data presented herein indicate that as much as 65% of the variability in mean suspended sediment concentration in the breaker zone can be accounted for with a marginal degree of certainty. It is concluded, therefore, that sediment suspension by breaking waves is not a random process.

The principal controlling factors of sediment concentration at sites similar to Price Inlet, South Carolina (gently sloping, fine-sand beaches, densely compacted) are, in order of importance:

1. Elevation above the bed.
2. Breaker type.
3. Distance relative to the breakpoint.
4. Beach slope.
5. Wave height.

Such process variables as wave period, longshore current velocity and wind velocity have little or no effect on concentration in the range of moderate swell conditions sampled.

With regard to the above, the following conclusions are offered:

1. Concentration decreases exponentially above the bottom to approximately 60 cm elevation as a function of the intermittent suspension of coarse sediment from the bed.
2. Relative breaker height (d_b/H_b) is useful for quantifying breaker types and predicting concentration at a given elevation.
3. In spilling waves, concentration gradually increases inside the breakpoint, then remains relatively constant under the bore as it propagates toward the beach. In plunging waves, concen-

tration peaks within a few meters of the breakpoint, then decreases gradually toward shore.

The final conclusion inferred from these data is that sediment transport is highly dependent on breaker type. Net offshore movement and equilibrium profiles can be qualitatively explained on the basis of variations in wave form, beach slope and suspended sediment concentration. Longshore transport rates are dependent not only on wave height, but on breaker type, which can be quantified to reasonable certainty by the simple ratio, d_b/H_b , relative wave height.

REFERENCES CITED

- Airy, G.B., 1845, Tides and waves: Encyc. Metrop. Art., 192, 241-396.
- Anan, Fayez S., 1972, Hydraulic equivalent sediment analyzer (HESA): Tech. Rept. No. 3-CRC, Coastal Research Center, Univ. of Massachusetts, 38p.
- Bagnold, R.A., 1947, Sand movement by waves: some small scale experiments with sand at very low density: J. Inst. Civ. Engrs., 27, No. 4, p. 447-69.
- Barr, A.J., Goodnight, J. H., Sall, J.P., and Helwig, J.T., 1976, A user's guide to SAS76: SAS Institute, Inc., Raleigh, North Carolina, 329p.
- Basinski, T. and Lewandowsky, A., 1974, Field investigations of suspended sediment: Proc. 14th Conf. on Coastal Engr., A.S.C.E., p. 1096-1108.
- Battjes, J.A., 1974, Surf similarity: Proc. 14th Conf. on Coastal Engr., A.S.C.E., p. 466-480.
- Boussinesq, J., 1872, Theorie des ondes et de remous qui se propagent le long d'un canal rectangulaire horizontal, en communiquant au liquide contenu dans ce canal des vitesses sensiblement paralleles de la surface au fond: J. Math. Pures et Appliquees (Lionville, France), 17, p. 55-108.
- Brennkmeier, B.M., 1973, Synoptic surf zone sedimentation patterns: Ph.D. Dissertation, Univ. of Southern California, 274p.
- _____, 1974, Sediment concentration changes in the surf zone: Mem. Inst. Geol. Bassin Aquitaine, no. 7, p. 115-118.
- _____, 1976a, In situ measurements of rapidly fluctuating, high sediment concentrations: Marine Geology, 20, p. 117-128.
- _____, 1976b, Sand fountains in the surf zone: in Davis, R.A., Jr. and Ethington, R., (eds.), Beach and Nearshore Sedimentation, S.E.P.M. Spec. Pub. 24, p. 69-91.
- Coakley, J. P., Savile, H.A., Pedrosa, M., and Larocque, M., 1978, Sled system for profiling suspended littoral drift: Proc. 16th Conf. on Coastal Engr., A.S.C.E., Hamburg, West Germany.
- Das, M., 1972, Mechanics of sediment suspension due to oscillatory water waves: in Shen, H., (ed.), Sedimentation, Symposium Volume, Fort Collins, Colorado 80521, p. 11-1 to 11-23.
- Dean, R.G., 1973, Heuristic models of sand transport in the surf zone: Proc. Conf. on Engineering Dynamics in the Surf Zone, Sydney, Australia, p. 208-214.

- Emery, K. O., 1961, A simple method of measuring beach profiles: *Limnology and Oceanography*, v. 6, p. 90-93.
- Fairchild, J.C., 1972, Longshore transport of suspended sediment: *Proc. 13th Conf. on Coastal Engr., A.S.C.E.*, p. 1062-1088.
- _____, 1977, Suspended sediment in the littoral zone at Ventnor, New Jersey and Nags Head, North Carolina: C.E.R.C. Tech. Paper No. 77-5, U.S. Army Corps of Engrs., 97p.
- Finley, R.J., 1976, Hydraulics and dynamics, North Inlet, South Carolina, 1974-1975: GITI Rept No. 10, Coastal Engineering Research Center, U.S. Army Corps of Engineers, Washington, D. C., 188p.
- Fürhböter, A., 1970, Air entrainment and energy dissipation in breakers: *Proc. 12th Conf. on Coastal Engr., A.S.C.E.*, p. 381-398.
- Galvin, C. J., Jr., 1968, Breaker type classification on three laboratory beaches: *J. of Geophys. Res.*, 73(12), p. 3651-3659.
- _____, 1972, Wave breaking in shallow water: in Meyer, R., (ed.), *Waves on Beaches and Resulting Sediment Transport*, Academic Press, New York, N. Y., p. 413-456.
- Hattori, M., 1969, The mechanics of suspended sediment due to standing waves: *Coastal Engr. in Japan*, 12, p. 69-81.
- _____, 1971, A further investigation of the distribution of suspended sediment concentration due to standing waves: *Coastal Engr. in Japan*, 14, p. 73-82.
- Hom-ma, M., Horikawa, K., and Kajima, R., 1965, A study on suspended sediment due to wave action: *Coastal Engr. in Japan*, 8, p. 85-103.
- Horikawa, K. and Watanabe, A., 1970, Turbulence and sediment concentration due to waves: *Proc. 12th Conf. on Coastal Engr., A.S.C.E.*, p. 751-766.
- Inman, D. L., 1977, Status of surf zone sediment transport relations: *Proc. Workshop on Coastal Sediment Transport with Emphasis on the National Sediment Transport Study*, manuscript, 12p.
- Ippen, A., and Kulin, G., 1955, The shoaling and breaking of the solitary wave: *Proc. 5th Conf. Coast Eng., A.S.C.E.*, p. 27-49.
- Iverson, H.W., 1952, Waves and breakers in shoaling water: *Proc. Third Conf. Coastal Eng., A.S.C.E.*, p. 1-12.

Johnson, J.W., 1949, Scale effects in hydraulic models involving wave motion: Transactions, Am. Geophys. Union, 30, p. 517-525.

Kana, T. W., 1976a, Sediment transport rates and littoral processes near Price Inlet, S.C.: in Hayes, M.O. and Kana, T.W., (eds.), Terrigenous Clastic Depositional Environments, CRD-Tech. Rept. No. 11, Dept. of Geology, Univ. of South Carolina, p. II-158 to II-171.

_____, 1976b, A new apparatus for collecting simultaneous water samples in the surf zone: J. Sed. Petrology, 46 (4), p. 1031-34.

_____, 1977, Suspended sediment transport at Price Inlet, S. C.: Proc. Coastal Sediments '77, A.S.C.E., Charleston, S. C., p. 366-382.

_____, and Ward, L. G., in prep., Suspended sediment and long-shore transport at Duck, N. C.: Final Report on Contract No. DACW72-78-M-0865, Coast. Eng. Research Center, U.S. Army Corps of Engrs., Washington, D. C.

Kennedy, J.F. and Locher, F. A., 1972, Sediment suspension by water waves: in Meyer, R. E. (ed.), Waves on Beaches and Resulting Sediment Transport, Academic Press, New York, p. 249-295.

King, C.A.M., and Williams, W., 1949, The formation and movement of sand bars by wave action: Geog. Journal, 113, p. 70-85.

Komar, P.D., 1978, Relative quantities of suspension versus bed-load transport on beaches: J. Sedim. Petrol., 48, No. 3, p. 921-932.

Leonard, J.E. and Brenninkmeyer, B.M., 1978, Periodicity of suspended sand movement during a storm: Proc. 16th Conf. on Coastal Engr., A.S.C.E., Hamburg, West Germany.

Locher, F.A., Glover, J.R., and Nakato, T., 1976, Investigation of the operating characteristics of the Iowa Sediment Concentration Measuring Systems: Tech. Paper No. 76-6, Coast. Eng. Research Center, U.S. Army Corps of Engineers, 99p.

MacDonald, T.C., 1977, Sediment suspension and turbulence in an oscillating flume: C.E.R.C. Tech. Paper No. 77-4, U.S. Army Corps of Engrs., 80p.

McCowan, J., 1894, On the highest wave of permanent type: Phil. Mag., series 5, 38, p. 351-57.

Miller, R.L., 1976, Role of vortices in surf zone prediction: sedimentation and wave forces: in Davis, R.A., Jr. and Ethington, R., (eds.), Beach and Nearshore Sedimentation, S.E.P.M. Spec. Pub. No. 24, p. 92-114.

- Munk, W.H., 1949, The solitary wave theory and its application to surf problems: Annals of the N.Y. Acad. of Sciences, 51(3), p. 376-424.
- Nakato, T., Locher, F.A., Glover, J.R., and Kennedy, J., 1977, Wave entrainment of sediment from rippled beds: J. of the Waterway, Port, Coastal and Ocean Div., A.S.C.E., 103, No. WW1, p. 83-99.
- Nummedal, D. and Stephen, M., 1976, Coastal dynamics and sediment transportation, Northeast Gulf of Alaska: Tech. Report No. 9-CRD, Coastal Research Division, Univ. of South Carolina, 148p.
- Patrick, D.A. and Wiegel, R.L., 1955, Amphibian tractors in the surf: Proc. 1st Conf. Ships and Waves, Council Wave Res., A.S.C.E., p. 397-422.
- Watts, G.M., 1953, Development and field tests of a sampler for suspended sediment in wave action: Beach Erosion Board Tech. Memo. No. 34, U.S. Army Corps of Engrs., 41p.

APPENDIX A - Photo Atlas of Waves Sampled and Corresponding Suspended
Sediment Data; Ordered by Breaker Type and Wave Height.

Contents:

<u>Breaker Type</u>	<u>Wave Height</u> (cm)	<u>Page</u>
Spilling	40	101
	45	102
	65	103
	70	104
	75	105
	80	106
	85	107
	90	108
	95	109
	110	110
	120	111
	125	112
	140	113
	145	114
Transition	70	115
	75	116
	80	117
	90	118
	95	119
	105	120
	115	121
Plunging	25	122
	45	123
	70	124
	80	125
	85	127
	90	129
	95	131
	100	132
	115	133

MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 40 cm
 Depth at Breaking (d) 47 cm
 d/H_b 1.167
 n 9

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.038</u>	<u>3</u>
30	<u>0.102</u>	<u>8</u>
10	<u>0.260</u>	<u>9</u>

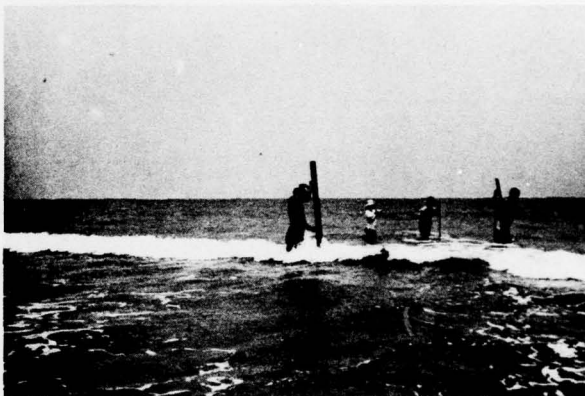
RESULTS - THIS PHOTO SERIES 60715

H_b 40 d 50
 Period 8.0 s α_b 0 °
 Longshore Current Velocity 0 cm/s
 Current Direction ----
 Wind Velocity 12 mph Azimuth 205 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>1</u>	100	
		60	
		30	<u>0.018</u>
		10	<u>0.040</u>
2	<u>7</u>	100	
		60	
		30	<u>0.035</u>
		10	<u>0.154</u>
3		100	
		60	
		30	
		10	

PHOTOS TAKEN AT

Station PI - 1
 Date 18 June, 1977
 Time 1359



MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 45 cm
 Depth at Breaking (d) 54 cm
 d/H_b 1.202
 n 11

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.076</u>	<u>3</u>
30	<u>0.112</u>	<u>8</u>
10	<u>0.238</u>	<u>11</u>

RESULTS - THIS PHOTO SERIES 60706

H_b 45 d 65
 Period 6.0 s α_b 0 °
 Longshore Current Velocity 0 cm/s
 Current Direction ---
 Wind Velocity 9 mph Azimuth 200 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.051</u>
		30	<u>0.150</u>
		10	<u>0.420</u>
2	<u>12</u>	100	
		60	
		30	<u>0.100</u>
		10	<u>0.370</u>
3		100	
		60	
		30	
		10	

PHOTOS TAKEN AT

Station PI - 1
 Date 18 June, 1977
 Time 1030

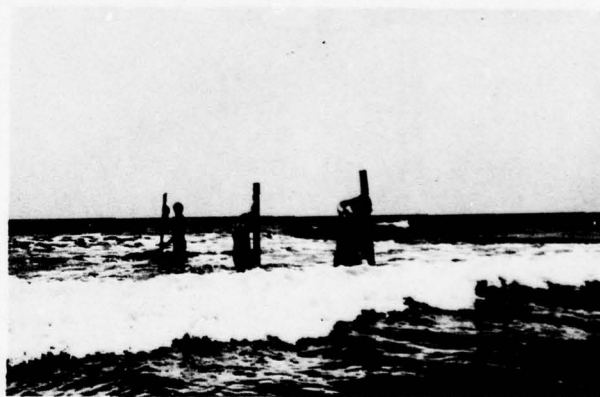
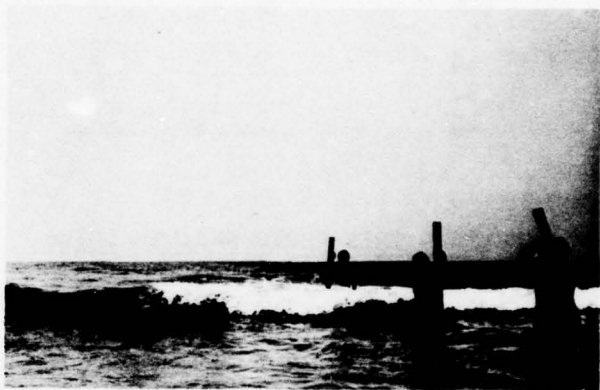
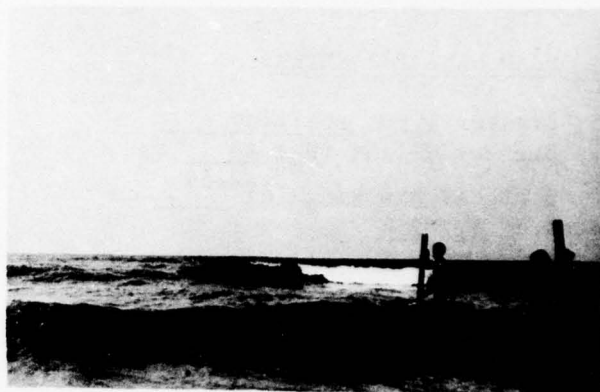


MEAN WAVE PARAMETERSBreaker Type spillingBreaker Height (H_b) 60 cmDepth at Breaking (d) 70 cm d/H_b 1.172n 14MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.033</u>	<u>13</u>
30	<u>0.083</u>	<u>6</u>
10	<u>0.169</u>	<u>14</u>

RESULTS - THIS PHOTO SERIES 60801 H_b 60 d 75Period 5.5 s α_b 8 °Longshore Current Velocity 31 cm/sCurrent Direction NorthWind Velocity 11 mph Azimuth 220 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.096</u>
		30	<u>0.143</u>
		10	<u>0.139</u>
2	<u>8</u>	100	
		60	
		30	<u>0.123</u>
		10	<u>0.129</u>
3		100	
		60	
		30	
		10	

PHOTOS TAKEN ATStation BU - 2Date 19 June, 1977Time 1108

MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 70 cm
 Depth at Breaking (d) 79 cm
 d/H_b 1.133
 n 7

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.026</u>	<u>7</u>
30	<u>0.027</u>	<u>2</u>
10	<u>0.289</u>	<u>7</u>

RESULTS - THIS PHOTO SERIES 60307

H_b 70 d 80
 Period 7.0 s \propto_b 5 °
 Longshore Current Velocity 25 cm/s
 Current Direction North
 Wind Velocity 4 mph Azimuth 120°

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>3</u>	100	
		60	<u>0.073</u>
		30	<u>0.115</u>
		10	<u>0.264</u>
2	3	100	
		60	
		30	<u>0.286</u>
		10	<u>0.294</u>
3	<u>3</u>	100	
		60	<u>0.032</u>
		30	
		10	<u>0.124</u>

PHOTOS TAKEN AT

Station PI - 1
 Date 13 June, 1977
 Time 1125



MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 75 cm
 Depth at Breaking (d) 85 cm
 d/H_b 1.133
 n 5

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.040</u>	<u>2</u>
30	<u>0.060</u>	<u>3</u>
10	<u>0.154</u>	<u>5</u>

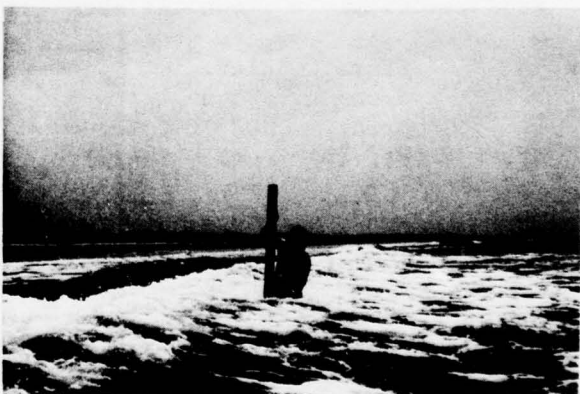
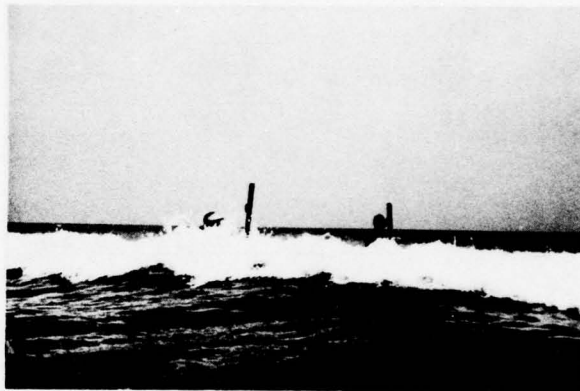
RESULTS - THIS PHOTO SERIES 60511

H_b 75 d 90
 Period 8.0 s ω_b 6 °
 Longshore Current Velocity 10 cm/s
 Current Direction South
 Wind Velocity 6 mph Azimuth 135 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>4</u>	100	
		60	
		30	<u>0.058</u>
		10	<u>0.052</u>
2	<u>7</u>	100	
		60	
		30	<u>0.023</u>
		10	<u>0.114</u>
3	<u>16</u>	100	
		60	
		30	<u>0.163</u>
		10	<u>0.337</u>

PHOTOS TAKEN AT

Station BU - 2
 Date 15 June, 1977
 Time 1410



MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 80 cm
 Depth at Breaking (d) 88 cm
 d/H_b 1.101
 n 21

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.098</u>	<u>12</u>
30	<u>0.151</u>	<u>19</u>
10	<u>0.369</u>	<u>21</u>

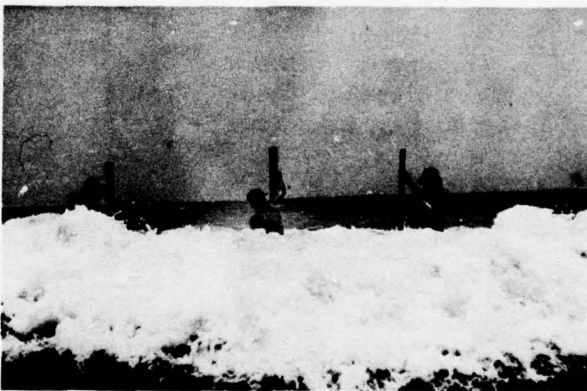
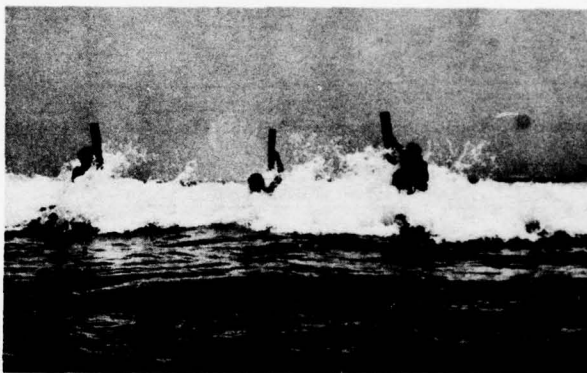
RESULTS - THIS PHOTO SERIES 61110

H_b 80 d 90
 Period 8.0 s α_b 0 °
 Longshore Current Velocity 0 cm/s
 Current Direction ---
 Wind Velocity 4 mph Azimuth 185 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.062</u>
		30	<u>0.125</u>
		10	<u>0.169</u>
2	<u>2</u>	100	
		60	
		30	<u>0.149</u>
		10	<u>0.547</u>
3	<u>2</u>	100	
		60	
		30	<u>0.141</u>
		10	<u>0.142</u>

PHOTOS TAKEN AT

Station BU - 2
 Date 28 June, 1977
 Time 1330

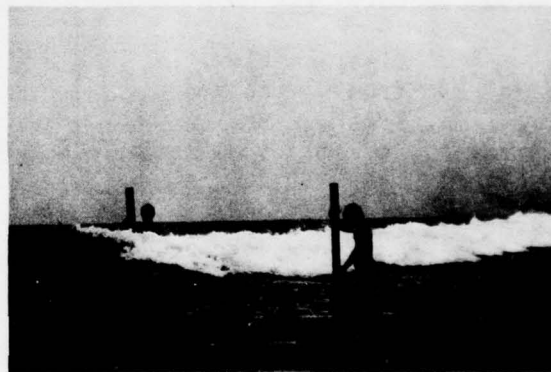
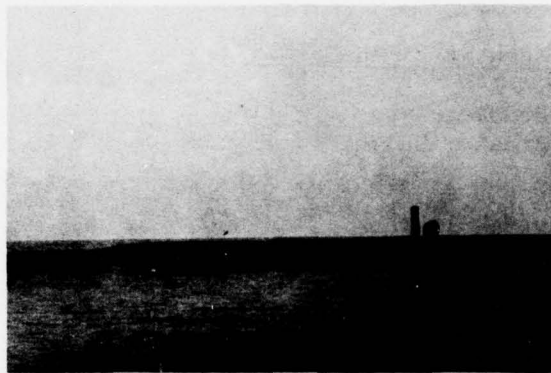


MEAN WAVE PARAMETERSBreaker Type spillingBreaker Height (H_b) 85 cmDepth at Breaking (d) 94 cm d/H_b 1.103n 4MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.074</u>	<u>2</u>
30	<u>0.121</u>	<u>4</u>
10	<u>0.448</u>	<u>4</u>

RESULTS - THIS PHOTO SERIES 61708 H_b 85 d 95Period 8.0 s α_b 0 °Longshore Current Velocity 0 cm/sCurrent Direction ---Wind Velocity 4 mph Azimuth 270 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>1</u>	100	
		60	<u>0.147</u>
		30	<u>0.207</u>
		10	<u>0.317</u>
2		100	
		60	
		30	
		10	
3		100	
		60	
		30	
		10	

PHOTOS TAKEN ATStation BU-2Date 5 July, 1977Time 1211

MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 90 cm
 Depth at Breaking (d) 99 cm
 d/H_b 1.103
 n 7

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.157</u>	<u>5</u>
30	<u>0.110</u>	<u>6</u>
10	<u>0.430</u>	<u>7</u>

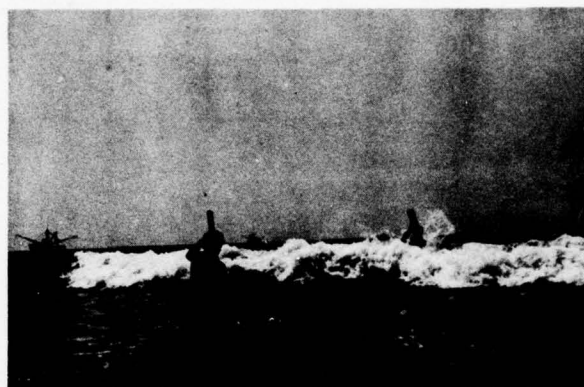
RESULTS - THIS PHOTO SERIES 61402

H_b 90 d 100
 Period 9.0 s α_b 3 °
 Longshore Current Velocity 60 cm/s
 Current Direction North
 Wind Velocity 13 mph Azimuth 210 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.087</u>
		30	<u>0.116</u>
		10	<u>0.176</u>
2	<u>7</u>	100	
		60	
		30	<u>0.120</u>
		10	<u>0.355</u>
3	<u>12</u>	100	
		60	
		30	<u>0.115</u>
		10	<u>0.237</u>

PHOTOS TAKEN AT

Station BU - 2
 Date 1 July, 1977
 Time 1145



MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 95 cm
 Depth at Breaking (d) 100 cm
 d/H_b 1.053
 n 10

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.375</u>	<u>5</u>
30	<u>0.215</u>	<u>9</u>
10	<u>0.511</u>	<u>10</u>

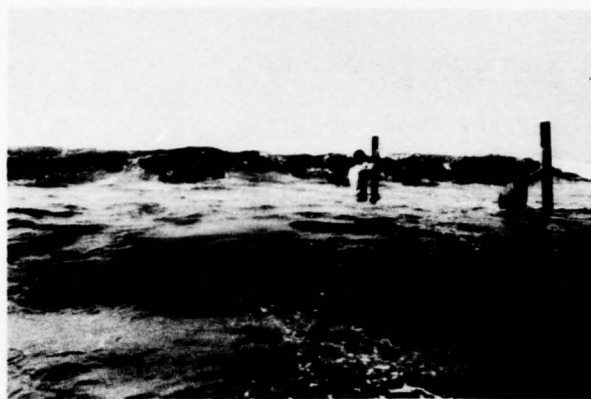
RESULTS - THIS PHOTO SERIES 60901

H_b 95 d 100
 Period 6.5 s α_b 8 °
 Longshore Current Velocity 45 cm/s
 Current Direction North
 Wind Velocity 7 mph Azimuth 220 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>0</u>	100	
		60	<u>0.033</u>
		30	<u>0.043</u>
		10	<u>0.082</u>
2	<u>5</u>	100	
		60	
		30	<u>0.160</u>
		10	<u>0.387</u>
3	<u>10</u>	100	
		60	
		30	<u>0.079</u>
		10	<u>1.548</u>

PHOTOS TAKEN AT

Station CA - 1
 Date 20 June, 1977
 Time 1012



MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 110 cm
 Depth at Breaking (d) 115 cm
 d/H_b 1.045
 n 4

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.098</u>	<u>3</u>
30	<u>0.091</u>	<u>4</u>
10	<u>0.151</u>	<u>4</u>

RESULTS - THIS PHOTO SERIES 60903

H_b 110 d 115
 Period 7.0 s α_b 4 °
 Longshore Current Velocity 45 cm/s
 Current Direction North
 Wind Velocity 7 mph Azimuth 220 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.055</u>
		30	<u>0.075</u>
		10	<u>0.089</u>
2	<u>9</u>	100	
		60	
		30	<u>0.147</u>
		10	<u>0.135</u>
3	<u>14</u>	100	
		60	
		30	<u>0.165</u>
		10	<u>0.172</u>

PHOTOS TAKEN AT

Station CA - 1
 Date 20 June, 1977
 Time 1035

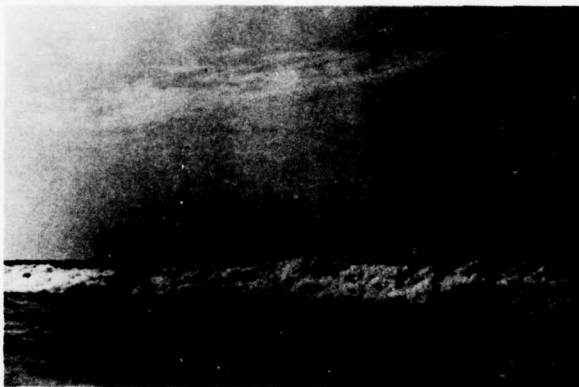
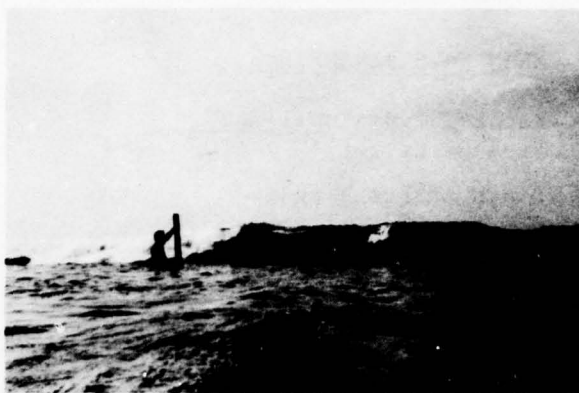


MEAN WAVE PARAMETERSBreaker Type spillingBreaker Height (H_b) 120 cmDepth at Breaking (d) 128 cm d/H_b 1.063n 4MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	<u>0.039</u>	<u>1</u>
60	<u>0.054</u>	<u>2</u>
30	<u>0.078</u>	<u>4</u>
10	<u>0.110</u>	<u>4</u>

RESULTS - THIS PHOTO SERIES 60902 H_b 120 d 130Period 6.5 s α_b 8 °Longshore Current Velocity 45 cm/sCurrent Direction NorthWind Velocity 7 mph Azimuth 220 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.029</u>
		30	<u>0.029</u>
		10	<u>0.069</u>
2	<u>7</u>	100	
		60	
		30	<u>0.072</u>
		10	<u>0.119</u>
3	<u>12</u>	100	
		60	
		30	<u>0.181</u>
		10	<u>0.225</u>

PHOTOS TAKEN ATStation CA - 1Date 20 June, 1977Time 1023

MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 125 cm
 Depth at Breaking (d) 140 cm
 d/H_b 1.120
 n 3

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	0.056	1
60	0.076	1
30	0.138	3
10	0.256	3

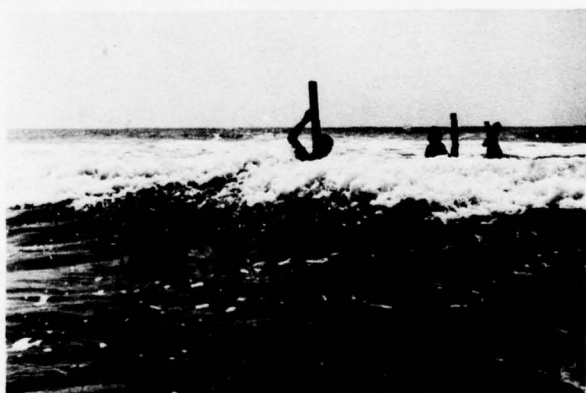
RESULTS - THIS PHOTO SERIES 61401

H_b 125 d 130
 Period 9.0 s $\propto \frac{1}{f}$ $\propto \frac{1}{n}$ 3 °
 Longshore Current Velocity 60 cm/s
 Current Direction North
 Wind Velocity 13 mph Azimuth 210 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>20</u>	100	
		60	<u>0.144</u>
		30	<u>0.114</u>
		10	<u>0.114</u>
2	<u>25</u>	100	
		60	
		30	<u>0.250</u>
		10	<u>0.385</u>
3	<u>30</u>	100	
		60	
		30	<u>0.149</u>
		10	<u>0.144</u>

PHOTOS TAKEN AT

Station BU - 2
 Date 1 July, 1977
 Time 1120



MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 140 cm
 Depth at Breaking (d) 150 cm
 d/H_b 1.07
 n 2

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	<u>.084</u>	<u>2</u>
60	<u>.103</u>	<u>2</u>
30	<u>.093</u>	<u>2</u>
10	<u>.180</u>	<u>2</u>

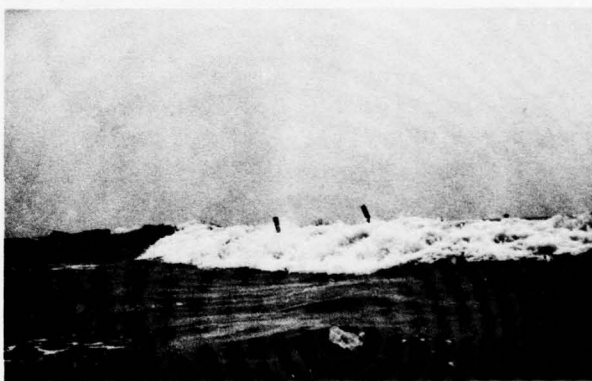
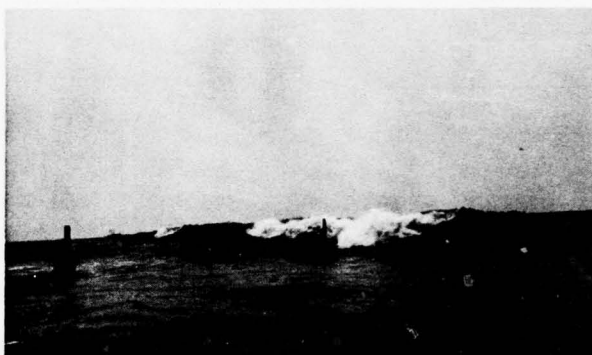
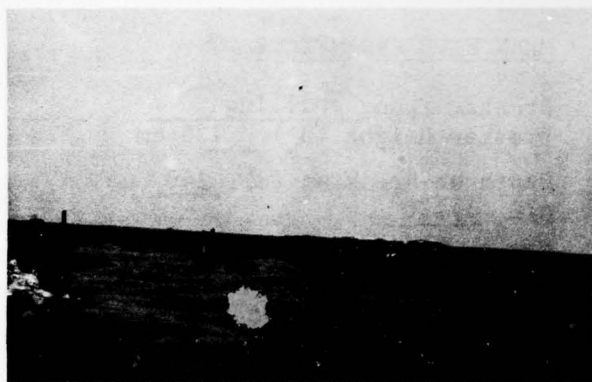
RESULTS - THIS PHOTO SERIES 61804

H_b 140 d 130 °
 Period 5.0 s α_b 2 °
 Longshore Current Velocity 18 cm/s
 Current Direction South
 Wind Velocity 10 mph Azimuth 220 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>3</u>	100	<u>0.048</u>
	—	60	<u>0.075</u>
	—	30	<u>0.080</u>
	—	10	<u>0.106</u>
2	<u>13</u>	100	—
	—	60	—
	—	30	<u>0.202</u>
	—	10	<u>0.362</u>
3	—	100	—
	—	60	—
	—	30	—
	—	10	—

PHOTOS TAKEN AT

Station CA-1
 Date 6 July, 1977
 Time 1202



MEAN WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 145 cm
 Depth at Breaking (d) 150 cm
 d/H_b 1.04
 n 2

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	<u>.066</u>	<u>1</u>
60	<u>.065</u>	<u>2</u>
30	<u>.081</u>	<u>2</u>
10	<u>.209</u>	<u>2</u>

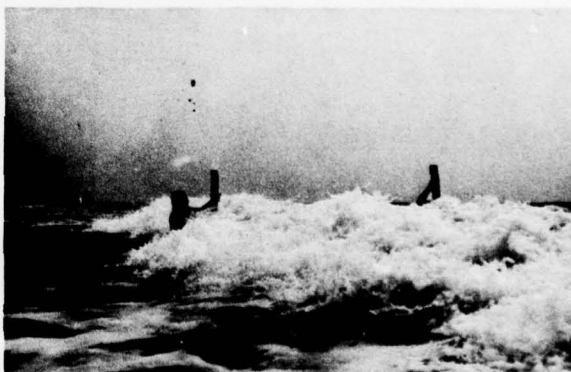
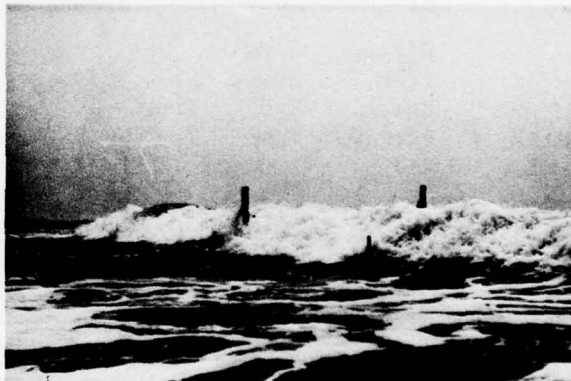
RESULTS - THIS PHOTO SERIES 61902

H_b 145 d 140
 Period 6.0 s α_b 5 °
 Longshore Current Velocity .32 cm/s
 Current Direction North
 Wind Velocity 9 mph Azimuth 215 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>8</u>	100	
		60	<u>0.067</u>
		30	<u>0.084</u>
		10	<u>0.342</u>
2	<u>13</u>	100	
		60	
		30	<u>0.067</u>
		10	<u>0.061</u>
3		100	
		60	
		30	
		10	

PHOTOS TAKEN AT

Station CA-1
 Date 7 July, 1977
 Time 1155



MEAN WAVE PARAMETERS

Breaker Type spill/plunge
 Breaker Height (H_b) 70 cm
 Depth at Breaking (d) 72 cm
 d/H_b 1.02
 n 50

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.115</u>	<u>22</u>
30	<u>0.460</u>	<u>39</u>
10	<u>0.781</u>	<u>50</u>

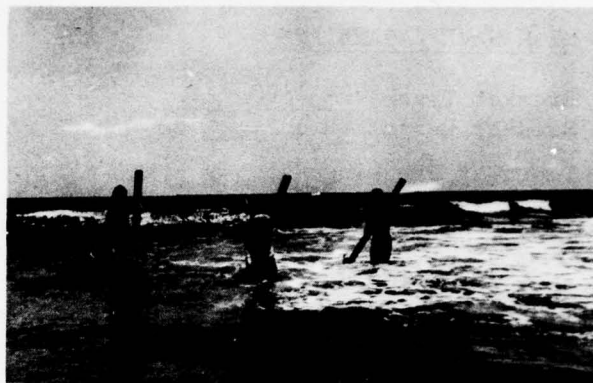
RESULTS - THIS PHOTO SERIES 61115

H_b 70 d 70
 Period 8.4 s α_b 3 °
 Longshore Current Velocity 33 cm/s
 Current Direction North
 Wind Velocity 10 mph Azimuth 200 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>3</u>	100	
		60	<u>0.455</u>
		30	<u>0.266</u>
		10	<u>1.134</u>
2	<u>3</u>	100	
		60	
		30	<u>0.345</u>
		10	<u>0.496</u>
3	<u>2</u>	100	
		60	
		30	<u>0.243</u>
		10	<u>0.951</u>

PHOTOS TAKEN AT

Station BU - 2
 Date 28 June, 1977
 Time 1423



MEAN WAVE PARAMETERS

Breaker Type spill/plunge
 Breaker Height (H_b) 75 cm
 Depth at Breaking (d) 68 cm
 d/H_b 0.910
 n 26

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.166</u>	<u>11</u>
30	<u>0.380</u>	<u>23</u>
10	<u>0.780</u>	<u>26</u>

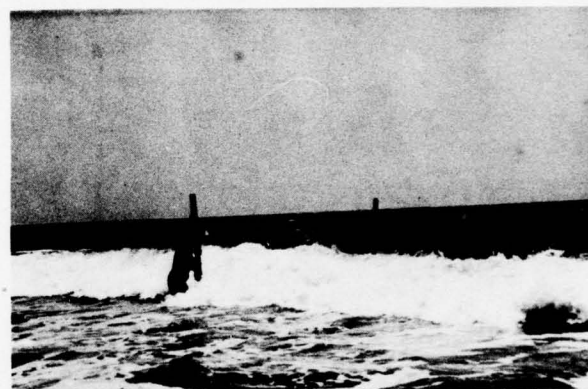
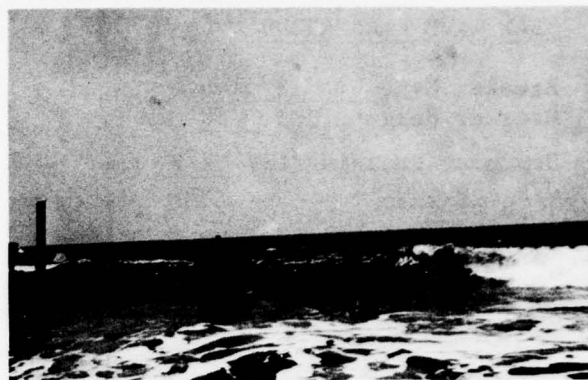
RESULTS - THIS PHOTO SERIES 61406

H_b 75 d 70
 Period 6.0 s α_b 5 °
 Longshore Current Velocity 33 cm/s
 Current Direction North
 Wind Velocity 18 mph Azimuth 210 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>0</u>	100	
		60	<u>0.334</u>
		30	<u>0.592</u>
		10	<u>0.698</u>
2	<u>5</u>	100	
		60	
		30	<u>0.837</u>
		10	<u>1.248</u>
3	<u>9</u>	100	
		60	
		30	<u>0.799</u>
		10	<u>0.982</u>

PHOTOS TAKEN AT

Station BU-2
 Date 1 July, 1977
 Time 1330



MEAN WAVE PARAMETERS

Breaker Type spill/plunge
 Breaker Height (H_b) 80 cm
 Depth at Breaking (d) 74 cm
 d/H_b 0.924
 n 57

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.166</u>	<u>29</u>
30	<u>0.380</u>	<u>51</u>
10	<u>1.018</u>	<u>57</u>

RESULTS - THIS PHOTO SERIES 60309

H_b 80 d 75
 Period 6.0 s α_b 5°
 Longshore Current Velocity 37 cm/s
 Current Direction North
 Wind Velocity 4 mph Azimuth 120°

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>5</u>	100	
		60	<u>0.137</u>
		30	<u>0.476</u>
		10	<u>0.820</u>
2	<u>5</u>	100	
		60	
		30	<u>0.372</u>
		10	<u>0.489</u>
3	<u>5</u>	100	
		60	<u>0.079</u>
		30	
		10	<u>0.141</u>

PHOTOS TAKEN AT

Station PI - 1
 Date 13 June, 1977
 Time 1153



MEAN WAVE PARAMETERS

Breaker Type spill/plunge
 Breaker Height (H_b) 90 cm
 Depth at Breaking (d) 83 cm
 d/H_b 0.922
 n 57

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.173</u>	<u>28</u>
30	<u>0.342</u>	<u>52</u>
10	<u>0.777</u>	<u>57</u>

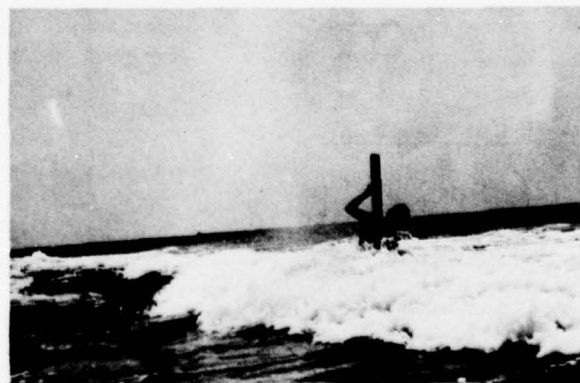
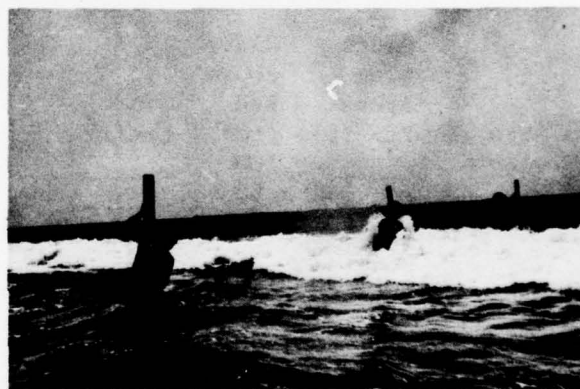
RESULTS - THIS PHOTO SERIES 61404

H_b 90 d 85
 Period 7.0 s α_b 3 °
 Longshore Current Velocity 10 cm/s
 Current Direction North
 Wind Velocity 13 mph Azimuth 210 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.009</u>
		30	<u>0.606</u>
		10	<u>0.565</u>
2	<u>7</u>	100	
		60	
		30	<u>0.555</u>
		10	<u>0.688</u>
3	<u>12</u>	100	
		60	
		30	<u>0.580</u>
		10	<u>0.604</u>

PHOTOS TAKEN AT

Station BU - 2
 Date 1 July, 1977
 Time 1220

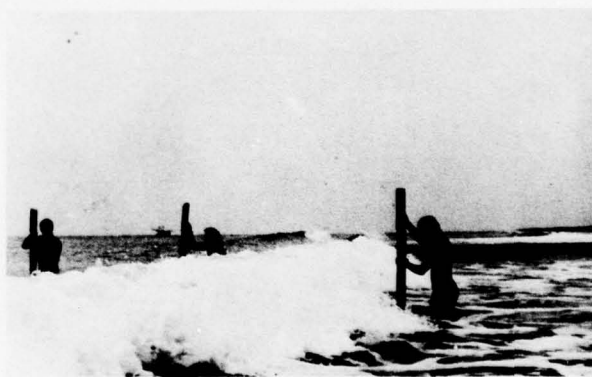
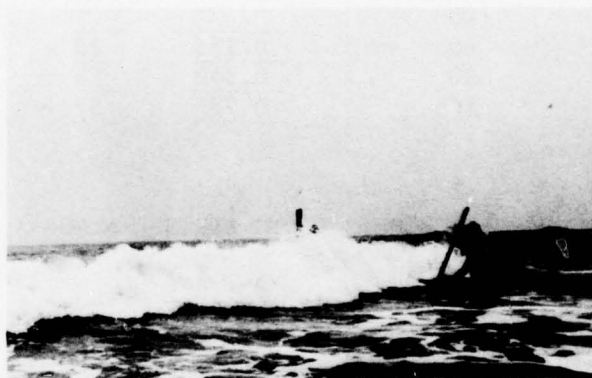
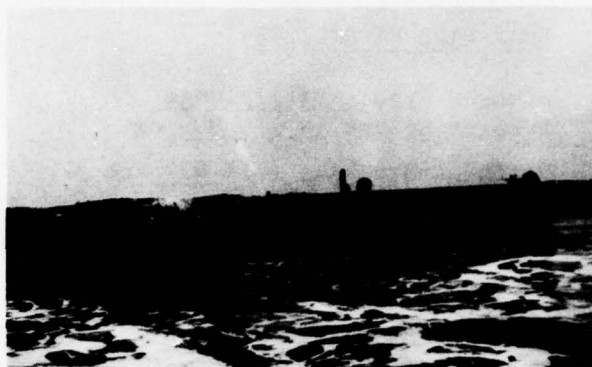


MEAN WAVE PARAMETERSBreaker Type spill/plungeBreaker Height (H_b) 95 cmDepth at Breaking (d) 91 cm d/H_b 0.961n 31MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.240</u>	<u>14</u>
30	<u>0.325</u>	<u>30</u>
10	<u>0.550</u>	<u>31</u>

RESULTS - THIS PHOTO SERIES 61707 H_b 95 d 85Period 7.5 s α_b 0°Longshore Current Velocity 0 cm/sCurrent Direction 0Wind Velocity 4 mph Azimuth 270°

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.080</u>
		30	<u>0.085</u>
		10	<u>0.127</u>
2	<u>4</u>	100	
		60	
		30	<u>0.131</u>
		10	<u>0.143</u>
3		100	
		60	
		30	
		10	

PHOTOS TAKEN ATStation BU-2Date 5 July, 1977Time 1105

MEAN WAVE PARAMETERS

Breaker Type spill/plunge
 Breaker Height (H_b) 105 cm
 Depth at Breaking (d) 103 cm
 d/H_b 0.976
 n 6

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	<u>0.051</u>	<u>1</u>
60	<u>0.100</u>	<u>3</u>
30	<u>0.425</u>	<u>6</u>
10	<u>0.998</u>	<u>6</u>

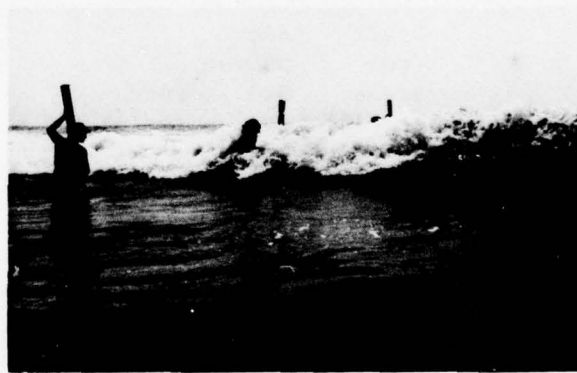
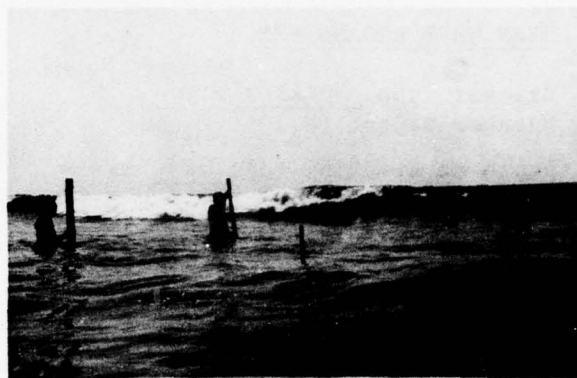
RESULTS - THIS PHOTO SERIES 61901

H_b 105 d 100
 Period 6.0 s α_b 5 °
 Longshore Current Velocity 32 cm/s
 Current Direction North
 Wind Velocity 9 mph Azimuth 215 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>3</u>	100	
		60	<u>0.079</u>
		30	<u>0.165</u>
		10	<u>1.381</u>
2	<u>8</u>	100	
		60	
		30	<u>1.452</u>
		10	<u>3.224</u>
3		100	
		60	
		30	
		10	

PHOTOS TAKEN AT

Station CA-1
 Date 7 July, 1977
 Time 1145



MEAN WAVE PARAMETERS

Breaker Type spill/plunge
 Breaker Height (H_b) 115 cm
 Depth at Breaking (d) 103 cm
 d/H_b 0.899
 n 9

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	0.090	2
60	0.115	3
30	0.219	9
10	0.819	9

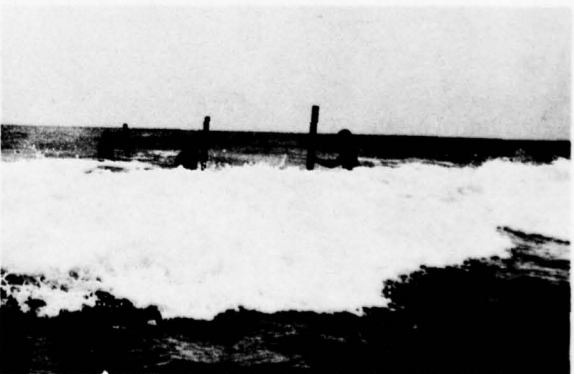
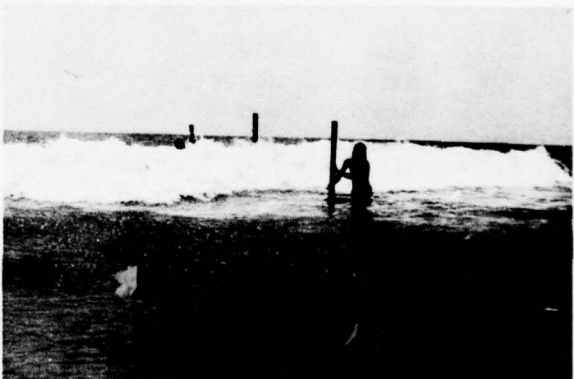
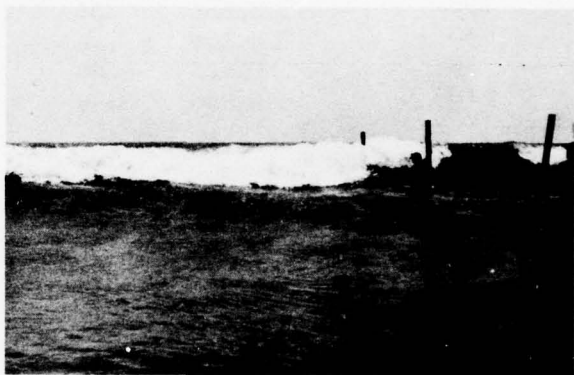
RESULTS - THIS PHOTO SERIES 61807

H_b 115 d 105
 Period 4.5 $s \propto_b$ 2 °
 Longshore Current Velocity 18 cm/s
 Current Direction South
 Wind Velocity 13 mph Azimuth 220 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	0	100	0.161
		60	0.198
		30	0.213
		10	0.246
2	4	100	
		60	
		30	0.595
		10	0.709
3	8	100	
		60	
		30	0.337
		10	0.507

PHOTOS TAKEN AT

Station CA-1
 Date 6 July, 1977
 Time _____



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 25 cm
 Depth at Breaking (d) 19 cm
 d/H_b 0.775
 n 8

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	_____	_____
60	_____	_____
30	<u>0.722</u>	<u>3</u>
10	<u>2.227</u>	<u>8</u>

RESULTS - THIS PHOTO SERIES 61108

H_b 25 d 20
 Period 6.0 s α_b 0°
 Longshore Current Velocity 0 cm/s
 Current Direction ---
 Wind Velocity 4 mph Azimuth 270°

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>0</u>	100	_____
	_____	60	_____
	_____	30	<u>0.543</u>
	_____	10	<u>1.469</u>
2	<u>3</u>	100	_____
	_____	60	_____
	_____	30	_____
	_____	10	<u>2.592</u>
3	<u>6</u>	100	_____
	_____	60	_____
	_____	30	_____
	_____	10	<u>6.154</u>

PHOTOS TAKEN AT

Station BU - 2
 Date 28 June, 1977
 Time 1207



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 45 cm
 Depth at Breaking (d) 30 cm
 d/H_b 0.667
 n 2

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60		
30	<u>2.574</u>	<u>2</u>
10	<u>7.497</u>	<u>2</u>

RESULTS - THIS PHOTO SERIES 60504

H_b 45 d 30
 Period 7.5 s α_b 2 °
 Longshore Current Velocity 10 cm/s
 Current Direction South
 Wind Velocity 2 mph Azimuth 330 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>3</u>	100	
		60	
		30	<u>3.121</u>
		10	<u>9.215</u>
2	<u>3</u>	100	
		60	
		30	<u>2.026</u>
		10	<u>5.778</u>
3	<u>3</u>	100	
		60	
		30	<u>10.221</u>
		10	<u>19.161</u>

PHOTOS TAKEN AT

Station BU - 2
 Date 15 June, 1977
 Time 1150



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 70 cm
 Depth at Breaking (d) 58 cm
 d/H_b 0.826
 n 43

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.156</u>	<u>15</u>
30	<u>0.500</u>	<u>37</u>
10	<u>0.861</u>	<u>43</u>

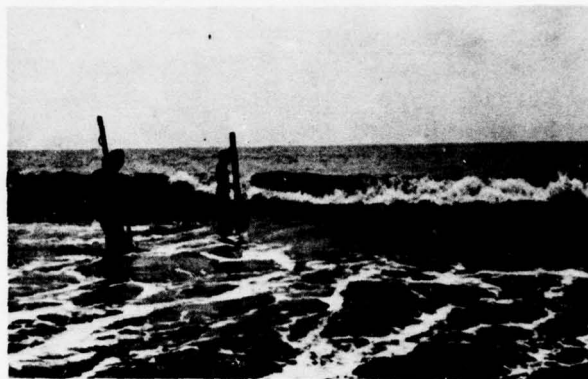
RESULTS - THIS PHOTO SERIES 61308

H_b 70 d 55
 Period 6.0 s α_b 5 °
 Longshore Current Velocity 24 cm/s
 Current Direction North
 Wind Velocity 7 mph Azimuth 240 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.398</u>
		30	<u>1.603</u>
		10	<u>0.886</u>
2	<u>9</u>	100	
		60	
		30	<u>2.255</u>
		10	<u>3.292</u>
3		100	
		60	
		30	
		10	

PHOTOS TAKEN AT

Station CA - 1
 Date 30 June, 1977
 Time 1258



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 80 cm
 Depth at Breaking (d) 65 cm
 d/H_b 0.811
 n 34

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.220</u>	<u>16</u>
30	<u>0.541</u>	<u>30</u>
10	<u>1.441</u>	<u>34</u>

RESULTS - THIS PHOTO SERIES 61212

H_b 80 d 55
 Period 7.2 s α_b 6 °
 Longshore Current Velocity 40 cm/s
 Current Direction North
 Wind Velocity 14 mph Azimuth 270 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>1</u>	100	
		60	<u>0.323</u>
		30	<u>1.520</u>
		10	<u>7.519</u>
2	<u>3</u>	100	
		60	
		30	<u>1.243</u>
		10	<u>4.378</u>
3	<u>5</u>	100	
		60	
		30	<u>0.719</u>
		10	<u>0.600</u>

PHOTOS TAKEN AT

Station CA - 1
 Date 29 June, 1977
 Time 1235



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 80 cm
 Depth at Breaking (d) 65 cm
 d/H_b 0.811
 n 34

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.220</u>	<u>16</u>
30	<u>0.542</u>	<u>30</u>
10	<u>1.441</u>	<u>34</u>

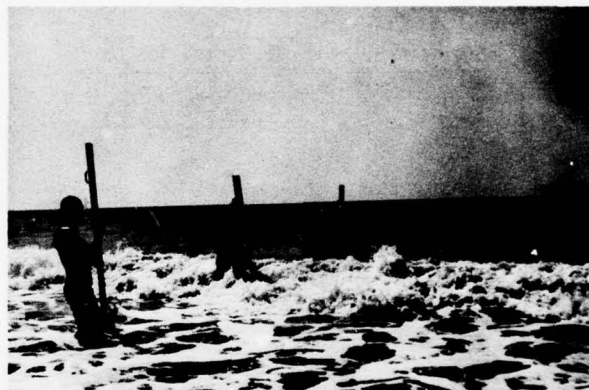
RESULTS - THIS PHOTO SERIES 61206

H_b 80 d 55
 Period 8.0 s α_b 10 °
 Longshore Current Velocity 40 cm/s
 Current Direction North
 Wind Velocity 5 mph Azimuth 260 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	0	100	
		60	<u>0.113</u>
		30	<u>0.509</u>
		10	<u>0.187</u>
2	4	100	
		60	
		30	<u>0.928</u>
		10	<u>0.187</u>
3	8	100	
		60	
		30	<u>1.472</u>
		10	<u>1.599</u>

PHOTOS TAKEN AT

Station CA - 1
 Date 29 June, 1977
 Time 1042



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 85 cm
 Depth at Breaking (d) 60 cm
 d/H_b 0.701
 n 13

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.305</u>	<u>6</u>
30	<u>1.021</u>	<u>10</u>
10	<u>3.285</u>	<u>13</u>

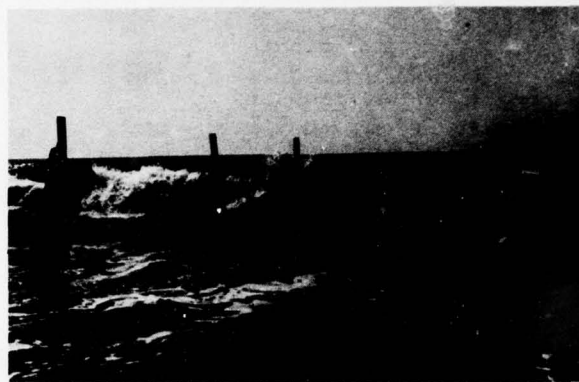
RESULTS - THIS PHOTO SERIES 61205

H_b 85 d 60
 Period 8.0 s α_b 10 °
 Longshore Current Velocity 40 cm/s
 Current Direction North
 Wind Velocity 5 mph Azimuth 260 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>0</u>	100	
		60	<u>0.116</u>
		30	<u>0.080</u>
		10	<u>0.254</u>
2	<u>3</u>	100	
		60	
		30	<u>0.274</u>
		10	<u>1.138</u>
3	<u>10</u>	100	
		60	
		30	<u>1.084</u>
		10	<u>3.323</u>

PHOTOS TAKEN AT

Station CA - 1
 Date 29 June, 1977
 Time 1030



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 85 cm
 Depth at Breaking (d) 60 cm
 d/H_b 0.701
 n 13

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.305</u>	<u>6</u>
30	<u>1.021</u>	<u>10</u>
10	<u>3.285</u>	<u>13</u>

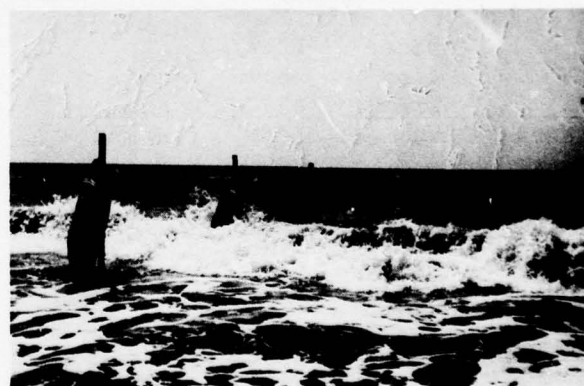
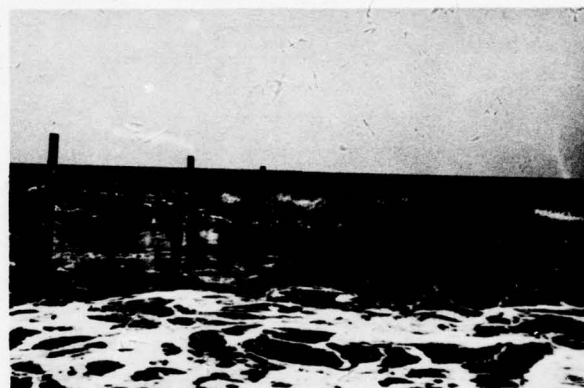
RESULTS - THIS PHOTO SERIES 61209

H_b 85 d 55
 Period 8.0 s α_b 10 °
 Longshore Current Velocity 40 cm/s
 Current Direction North
 Wind Velocity 5 mph Azimuth 260 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>1</u>	100	
		60	<u>0.513</u>
		30	<u>2.098</u>
		10	<u>6.690</u>
2	<u>4</u>	100	
		60	
		30	<u>2.875</u>
		10	<u>6.519</u>
3	<u>7</u>	100	
		60	
		30	
		10	<u>1.141</u>

PHOTOS TAKEN AT

Station CA - 1
 Date 29 June, 1977
 Time 1125



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 90 cm
 Depth at Breaking (d) 74 cm
 d/H_b 0.822
 n 24

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.228</u>	<u>11</u>
30	<u>0.486</u>	<u>20</u>
10	<u>1.277</u>	<u>24</u>

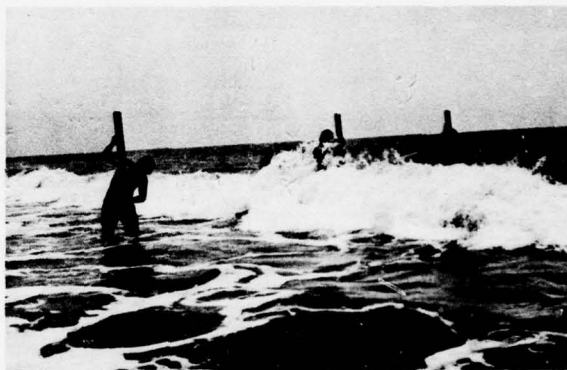
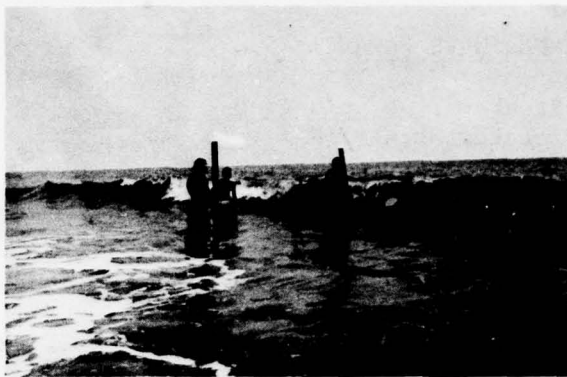
RESULTS - THIS PHOTO SERIES 61508

H_b 90 d 60
 Period 10.0 s α_b 4 °
 Longshore Current Velocity 21 cm/s
 Current Direction North
 Wind Velocity 4 mph Azimuth 230 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.261</u>
		30	<u>0.289</u>
		10	<u>2.248</u>
2	<u>4</u>	100	
		60	
		30	<u>0.738</u>
		10	<u>3.036</u>
3	<u>7</u>	100	
		60	
		30	<u>1.680</u>
		10	<u>2.218</u>

PHOTOS TAKEN AT

Station CA-1
 Date 2 July, 1977
 Time 1409



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 90 cm
 Depth at Breaking (d) 74 cm
 d/H_b 0.822
 n 24

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.228</u>	<u>11</u>
30	<u>0.486</u>	<u>20</u>
10	<u>1.277</u>	<u>24</u>

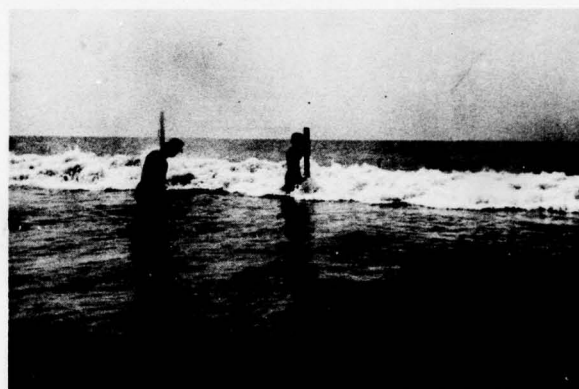
RESULTS -- THIS PHOTO SERIES -- 61303

H_b 90 d 80
 Period 8.0 s α_b 4 °
 Longshore Current Velocity 26 cm/s
 Current Direction North
 Wind Velocity 7 mph Azimuth 240 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>5</u>	100	
		60	<u>0.057</u>
		30	<u>0.081</u>
		10	<u>0.320</u>
2	<u>16</u>	100	
		60	
		30	<u>0.359</u>
		10	<u>0.797</u>
3		100	
		60	
		30	
		10	

PHOTOS TAKEN AT

Station CA - 1
 Date 30 June, 1977
 Time 1103



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 95 cm
 Depth at Breaking (d) 78 cm
 d/H_b 0.816
 n 8

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100		
60	<u>0.130</u>	<u>3</u>
30	<u>0.549</u>	<u>8</u>
10	<u>0.837</u>	<u>8</u>

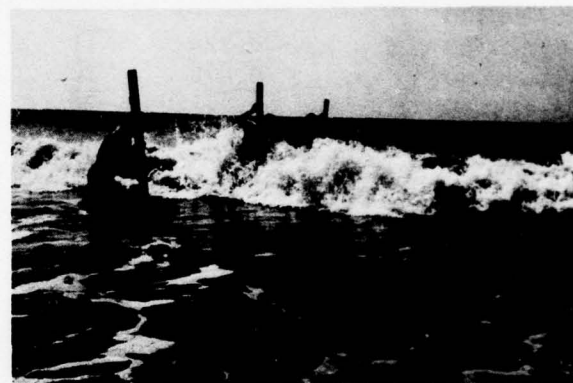
RESULTS - THIS PHOTO SERIES 61204

H_b 95 d 65
 Period 8.0 s \propto_b 10 °
 Longshore Current Velocity 40 cm/s
 Current Direction North
 Wind Velocity 5 mph Azimuth 260 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>1</u>	100	
		60	<u>0.170</u>
		30	<u>2.802</u>
		10	<u>3.382</u>
2	<u>5</u>	100	
		60	
		30	<u>0.113</u>
		10	<u>0.103</u>
3	<u>8</u>	100	
		60	
		30	<u>0.502</u>
		10	<u>0.873</u>

PHOTOS TAKEN AT

Station CA - 1
 Date 29 June, 1977
 Time 1015



MEAN WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 100 cm
 Depth at Breaking (d) 85 cm
 d/H_b 0.845
 n 11

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	<u>0.110</u>	<u>1</u>
60	<u>0.267</u>	<u>4</u>
30	<u>0.492</u>	<u>11</u>
10	<u>0.873</u>	<u>9</u>

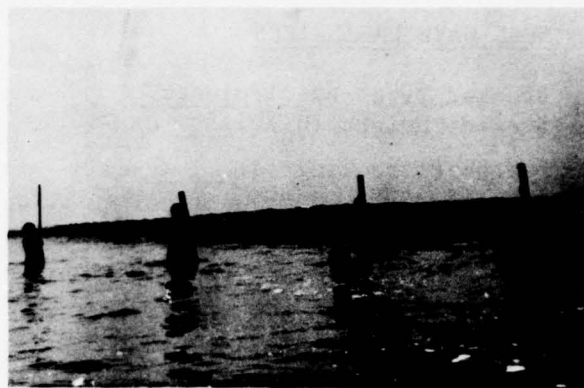
RESULTS - THIS PHOTO SERIES 61506

H_b 100 d 90
 Period 9.0 s α_b 4 °
 Longshore Current Velocity 21 cm/s
 Current Direction North
 Wind Velocity 4 mph Azimuth 230 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>5</u>	100	
		60	<u>0.211</u>
		30	<u>0.392</u>
		10	<u>0.601</u>
2	<u>5</u>	100	
		60	
		30	<u>0.489</u>
		10	<u>0.665</u>
3	<u>5</u>	100	
		60	
		30	<u>0.350</u>
		10	

PHOTOS TAKEN AT

Station CA-1
 Date 2 July, 1977
 Time 1235



MEAN WAVE PARAMETERS

Breaker Type spill/plunge
 Breaker Height (H_b) 115 cm
 Depth at Breaking (d) 103 cm
 d/H_b 0.899
 n 9

MEAN SUSPENDED SEDIMENT

Sample Elev. (cm)	Conc. (gm/l)	n
100	<u>0.090</u>	<u>2</u>
60	<u>0.115</u>	<u>3</u>
30	<u>0.219</u>	<u>9</u>
10	<u>0.819</u>	<u>9</u>

RESULTS - THIS PHOTO SERIES 61501

H_b 115 d 95
 Period 13.0 s α_b 0 °
 Longshore Current Velocity 0 cm/s
 Current Direction ---
 Wind Velocity 2 mph Azimuth 230 °

Array #	Dist. from Break. Pt. (m)	Elev. above Bed (cm)	Conc. (gm/l)
1	<u>2</u>	100	
		60	<u>0.086</u>
		30	<u>0.109</u>
		10	<u>0.328</u>
2	<u>3</u>	100	
		60	
		30	<u>0.286</u>
		10	<u>1.106</u>
3	<u>4</u>	100	
		60	
		30	<u>0.111</u>
		10	<u>0.158</u>

PHOTOS TAKEN AT

Station CA-1
 Date 2 July, 1977
 Time 1122



- APPENDIX B - 1. List of Computer Codes for Variables
 2. Example SAS76 Program Statements and Job Control Cards
 3. Data Printout for Selected Observations and Variables

1. Computer Codes

Field Generated Variables

<u>Code</u>	<u>Definition</u>
OBS	OBSERVATION NUMBER - computer generated
SERIES	NUMBER ASSIGNED - to each wave sampled
OPERATOR	IDENTIFYER - name of sampler operator
STA	STATION IDENTIFICATION
DATE	DATE - month, day, year
TIME	TIME - 24 hour system
WINDVEL	WIND VELOCITY - mph
WINDAZI	WIND AZIMUTH - degrees
WVHT	BREAKER HEIGHT - cm
WVDPH	BREAKER DEPTH - cm
PER	WAVE PERIOD - seconds
WVTYPE	VISUAL WAVE TYPE - 1=spilling 2=plunging 3=transition 4=non-breaking
LSCUR	LONGSHORE CURRENT VELOCITY - cm/s
LSDIR	LONGSHORE CURRENT DIRECTION - 1=to left 2=to right, viewed from shore
ALPHAB	BREAKER ANGLE - degrees
MOFF	AVERAGE BEACH SLOPE - seaward of breakpt.
STKDIST	DISTANCE FROM BENCHMARK ON LAND - to sample position
CRSTDIS	DISTANCE FROM BREAKPOINT - (+) is landward and (-) is seaward of breakers
BOREHT	HEIGHT OF WAVE BORE - at sampler, cm
BOREDPTH	DEPTH UNDER WAVE BORE - cm
SAMSLOPE	BEACH SLOPE AT SAMPLE POSITION
SAMTIME	SAMPLE TIME - with respect to phase of wave, seconds
SS10	SUSPENDED SEDIMENT CONCENTRATION AT 10 CM ABOVE THE BED - gm/l
SS30	"" "" 30 CM "" ""
SS60	"" "" 60 CM "" ""
SS100	"" "" 100 CM "" ""

Computer Generator Variables

D_OVERHB	RATIO WVDPH/WVHT - d_b/H_b
BRKER	PARAMETER - $(1-m)^4 d_b/H_b$
BB	GALVIN'S BREAKER PARAMETER - $H_b/(gmT^2)$
XI	BATTJES' SURF PARAMETER - $m/(H_b/L_0)^{1/2}$

B - 2. Example SAS76 Program Statements and Job Control Cards

```

//SS77 JOB (T3100203,5),'KANA',TIME=(,9),MSGLEVEL=(1,1),
// USER=T310020,PASS=WORD=XXXX
// EXEC SAS76
XASAS76 PROC OPTIONS=,SORT=4,ENTRY=SAS
*****
*** STATISTICAL ANALYSIS SYSTEM, VERSION 76.6 ***
*** DOCUMENTATION: A USER'S GUIDE TO SAS 76 ***
*** AND ***
*** SAS SUPPLEMENTAL LIBRARY ***
*** USER'S GUIDE ***
*****
XASAS EXEC PGM=ENTRY,PARM=,OPTIONS=,REGION=256K
XSLIBRARY DD DSN=ACAU,SAS766.LIBRARY,DISP=SHR,VOL=SER=ACAD00,UNIT=3350
XSTEMPLIB DD DSN=PLIX,PLITHYS,DISP=SHR,UNIT=3350,VOL=SER=SYSLNK
XX DD DSN=SMI,LINKLIB,DISP=SHR,UNIT=3350,VOL=SER=MVSP02
XX DD DSN=*,LIBRARY,DISP=(OLD,PASS),UNIT=SYSUA,VOL=REF=*,LIBRARY
XWORK DD UNIT=SYSUA,SPACE=(TRK,(240,80)) TEMPORARY DATA SETS
XFT11F001 DD SYSOUT=*,DCH=(BLKSIZE=141,LRECL=137,RECFM=VBA) LOG
XFT12F001 DD SYSOUT=*,DCH=(BLKSIZE=141,LRECL=137,RECFM=VBA) PRINT
XFT13F001 DD SYSOUT=D PUNCH
XFT13F001 DD UNIT=SYSUA,SPACE=(TRK,(10,10)),PARMCARDS
XX UCD=(RECFM=F3,LRECL=80,BLKSIZE=400,HUFNO=1)
*** SYSTEM SORT DEFINITIONS
XSORTLIB DD DSN=SMI,SORTLIB,DISP=SHR,UNIT=3350,VOL=SER=SYSLNK
XSSUUT DD SYSOUT=*,DCH=BUFNO=1
XSSORTWK01 DD SPACE=(CYL,(6,SORT)),UNIT=SYSUA
XSSORTWK02 DD SPACE=(CYL,(6,SORT)),UNIT=(SYSUA,,SEP=(SORTWK01))
XSSORTWK03 DD SPACE=(CYL,(6,SORT)),UNIT=(SYSUA,,SEP=(SORTWK01,
XX SORTWK02))
//DATAN DD DSN=T310020.SUSPEND,DISP=SHR
//SYSIN DD *
//

DATA SS77;
INFILE DATAN;
INPUT SERIES 1-5 SUBSERIS 6 OPERATOR SEXORDER ORIENT SECT STA $16-18 DATE TIME
TIME WINDVEL WINDAZI WVHT 41-43 WVDPTH 45-47 PER 49-52 WVTYPE 54 LSCUR 56-58
LSDIR 60 ALPHAB 62-63 ALPHADIR 65 MOFF 67-70 STRDIST 72-74 CRSTDIS 76-77 BOREHT
79-80 #2 BOREDPTH 1-2 SAMSLOPE 4-7 SAMTIME 9-10 SS10 12-17 SS30 19-24 SS60 26-
31 SS100 33-38 SS140 40-45;
D_OVERHB=WVDPTH/WVHT;
LOGSS10=LOG10(SS10);
LOGSS30=LOG10(SS30);
LOGSS60=LOG10(SS60);
LOGSS100=LOG10(SS100);
LGSS10=LOGSS10;
LGSS30=LOGSS30;
LGSS60=LOGSS60;
LGSS100=LOGSS100;
S10=SS10;
S30=SS30;
S60=SS60;
BH=WVHT/(980*MOFF*(PER**2));
BOKATIO=BOREDPTH/BOREHT;
MF4=(1-MOFF)**4;
BRKTYPE=D_OVERHB*MF4;
SAMSLS=(1-SAMSLOPE)**4;
BRKER=D_OVERHB*SAMSLS;
XI=MOFF/(6.28319*WVHT/980*(PER**2))**.5;
IF SERIES=6020; AND SUBSERIS=1 THEN DELETE;
IF SERIES=60202 AND SUBSERIS=1 THEN DELETE;
IF CRSTDIS<1 THEN DELETE;
IF CRSTDIS>12.5 THEN DELETE;
IF BRKTYPE>1.6 THEN DELETE;

PROC SORT; BY D_OVERHB;

DATA SET WORK.SS77 HAS 403 OBSERVATIONS AND 51 VARIABLES. 32 OBS/TRK.
THE PROCEDURE SORT USED 0.36 SECONDS AND 108K.

PROC MEANS NOPRINT; BY D_OVERHB;
VAR LGSS10 LGSS30 LGSS60;
OUTPUT OUT=NEWS52 MEAN=MLGSS10 MLGSS30 MLGSS60;

PROC PLOT;
PLOT MLGSS10 * D_OVERHB MLGSS30 * D_OVERHB MLGSS60 * D_OVERHB;
PROC CORR; VAR MLGSS10 WVHT D_OVERHB WVDPTH MOFF PER LSCUR WINDVEL SAMSLOPE;

THE PROCEDURE CORR USED 0.40 SECONDS AND 132K AND PRINTED PAGE 41.

SAS USED 152K MEMORY.

HARR: GOODNIGHT, SALL AND MELWIG
SAS INSTITUTE INC.
P.O. BOX 10066
RALEIGH, N.C. 27605

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		S		E		R		I		A		L		P		R		S		S	
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		S		E		R		I		A		L		P		R		S		S	
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		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
		S		E		R		I		A		L		P		R		S		S	
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424		61608		RUZ		70477		1354		80		70		10.0		0.87		0.813	
425		61608		RUZ		70477		1354		80		70		10.0		0.87		0.771	
426		61609		HUZ		70477		1405		75		75		7.0		1.00		0.878	
427		61609		HUZ		70477		1405		75		75		7.0		1.00		0.878	
428		61609		RUZ		70477		1405		75		75		7.0		1.00		0.929	
429		61610		HUZ		70477		1418		85		80		10.0		0.94		0.869	
430		61610		HUZ		70477		1418		85		80		10.0		0.94		0.826	
431		61611		RUZ		70477		1431		80		85		8.0		1.06		1.004	
432		61611		RUZ		70477		1431		80		85		8.0		1.06		1.004	
433		61611		HUZ		70477		1431		80		85		8.0		1.06		0.932	
434		61701		HUZ		70577		947		95		90		8.0		0.94		0.835	
435		61701		HUZ		70577		947		95		90		8.0		0.94		0.835	
436		61701		HUZ		70577		947		95		90		8.0		0.94		0.877	
437		61702		RUZ		70577		1000		90		85		8.0		0.94		0.832	
438		61702		RUZ		70577		1000		90		85		8.0		0.94		0.874	
439		61702		HUZ		70577		1000		90		85		8.0		0.94		0.856	
440		61703		RUZ		70577		1014		100		90		8.0		0.90		0.833	
441		61703		HUZ		70577		1014		100		90		8.0		0.90		0.816	
442		61703		RUZ		70577		1014		100		90		8.0		0.90		0.816	
443		61704		HUZ		70577		1026		100		95		8.0		0.95		0.862	
444		61704		RUZ		70577		1026		100		95		8.0		0.95		0.862	
445		61704		HUZ		70577		1026		100		95		8.0		0.95		0.908	
446		61705		RUZ		70577		1040		90		85		8.0		0.94		0.856	
447		61705		RUZ		70577		1040		90		85		8.0		0.94		0.903	
448		61705		HUZ		70577		1040		90		85		8.0		0.94		0.903	
449		61706		RUZ		70577		1058		90		85		8.0		0.94		0.963	
450		61706		HUZ		70577		1042		90		85		8.0		0.94		0.913	
451		61707		RUZ		70577		1105		95		85		7.5		0.89		0.856	
452		61707		HUZ		70577		1105		95		85		7.5		0.89		0.856	
453		61708		RUZ		70577		1211		85		80		8.0		0.94		0.960	
454		61801		CAI		70677		1104		130		130		5.0		1.00		0.956	
455		61801		CAI		70677		1104		130		130		5.0		1.00		0.949	
456		61801		CAI		70677		1104		130		130		5.0		1.00		0.968	
457		61802		CAI		70677		1125		145		135		5.0		0.93		0.876	
458		61802		CAI		70677		1125		145		135		5.0		0.93		0.824	
459		61802		CAI		70677		1125		145		135		5.0		0.93		0.867	
460		61803		CAI		70677		1143		120		125		5.0		1.04		0.980	
461		61803		CAI		70677		1143		120		125		5.0		1.04		0.922	
462		61804		CAI		70677		1202		140		140		5.0		0.92		0.822	
463		61804		CAI		70677		1202		140		140		5.0		0.92		0.859	
464		61805		CAI		70677		1215		120		110		5.0		0.91		0.811	
465		61805		CAI		70677		1215		120		110		5.0		0.91		0.811	
466		61805		CAI		70677		1215		120		110		5.0		0.91		0.888	
467		61806		CAI		70677		1230		120		115		5.0		0.95		0.947	
468		61806		CAI		70677		1230		120		115		5.0		0.95		0.925	
469		61807		CAI		70677		1244		115		105		4.5		0.91		0.845	
470		61807		CAI		70677		1244		115		105		4.5		0.91		0.945	

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518	62005	R02	70777	1222	3	64	1	3	0.022	.	0.469	.	.
519	62006	R02	70777	1210	3	64	1	3	0.022	.	1.757	0.996	.
520	62006	R02	70777	1230	3	64	1	3	0.022	.	1.417	.	.
521	62007	R02	70777	1218	1	62	1	3	0.022	.	0.181	.	.
522	62007	R02	70777	1234	1	62	1	3	0.022	.	0.710	.	.
523	62008	R02	70777	1250	140	135	5	0.96	0.452	0.259	0.0046	1	62	1	3	0.022	3	0.253	0.105	0.130
524	62102	CA1	70777	1116	85	78	6	0.82	0.732	0.068	0.0079	2	7	1	2	0.035	1	0.927	0.595	0.200
525	62102	CA1	70777	1114	85	70	6	0.82	0.732	0.068	0.0079	2	7	1	2	0.035	1	0.849	1.256	0.513
526	62105	CA1	70777	1210	100	95	6	0.95	0.931	0.113	0.0052	2	7	1	2	0.025	4	0.233	0.172	0.146
527	62105	CA1	70777	1210	100	95	6	0.95	0.901	0.113	0.0052	2	7	1	2	0.025	9	0.184	0.150	0.180
528	62105	CA1	70777	1210	100	95	6	0.95	0.890	0.113	0.0052	2	7	1	2	0.025	14	0.198	0.135	0.101

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APPENDIX C - 1. Suspended Sediment Grain Size Data - 1977.
 2. Representative Size Frequency Curves by Sample Position.

	<u>Code</u>	<u>Definition</u>
STA	STATION IDENTIFICATION	
DATE	DATE - month, day, year	
SERIES	NUMBER ASSIGNED - to each wave sampled	
WVHT	BREAKER HEIGHT - cm	
WVTYPE	VISUAL WAVE TYPE - 1=spilling 2=plunging 3=transition	
DIS	DISTANCE FROM BENCHMARK ON LAND - m	
HT	SAMPLE ELEVATION ABOVE BED - cm	
MZ	MEAN GRAIN SIZE - ϕ	
STDDEV	STANDARD DEVIATION - ϕ	
SKEW	SKEWNESS	
KURT	KURTOSIS	
SS	SUSPENDED SEDIMENT CONCENTRATION - g/l.	

OBS	STA	DATE	SERIES	WVHT	WVTYPE	DIS	HT	MZ	STDDEV	SKEW	KURT	SS
1	CA1	61477	604012	80	3	80	10	3.34	0.347	-0.170	-0.402	0.798
2	CA1	61477	604031	80	1	95	10	3.04	0.752	-0.754	2.439	.
3	CA1	61677	606073	45	1	105	30	3.13	0.443	-0.794	3.422	.
4	CA1	61677	606073	45	1	105	10	3.21	0.419	-0.490	5.551	.
5	PI1	61877	607023	40	1	55	10	3.02	0.649	-0.539	2.423	.
6	PI1	61877	607053	25	4	51	10	3.07	0.542	-0.402	1.184	.
7	CA1	62977	612052	55	2	108	30	3.32	0.342	-0.169	1.371	1.084
8	CA1	62977	612052	55	2	108	10	3.06	0.610	-0.877	4.671	3.323
9	CA1	62977	612062	45	2	114	30	3.34	0.346	-1.781	34.368	0.928
10	CA1	62977	612062	45	2	114	10	3.30	0.742	-0.210	0.487	1.067
11	CA1	62977	612063	35	2	110	30	3.30	0.242	0.370	0.382	1.472
12	CA1	62977	612063	35	2	110	10	3.31	0.292	0.091	0.411	1.599
13	CA1	62977	612072	35	2	115	10	3.08	0.922	-1.003	3.673	1.117
14	CA1	62977	612073	65	2	116	30	3.31	0.349	-0.408	2.989	1.172
15	CA1	62977	612073	65	2	116	10	3.01	0.749	-0.912	4.124	6.255
16	CA1	62977	612042	60	2	116	10	3.11	0.536	-0.690	1.622	2.579
17	CA1	62977	612043	50	2	115	30	2.91	0.711	-0.490	0.177	1.764
18	CA1	62977	612043	50	2	115	10	3.08	0.318	-0.052	0.470	9.026
19	CA1	62977	612092	35	2	118	10	3.25	0.438	-0.518	2.566	1.141
20	CA1	62977	612093	45	2	121	30	3.26	0.271	0.105	-0.579	2.875
21	CA1	62977	612093	45	2	121	10	3.11	0.577	-0.718	3.020	6.519
22	CA1	62977	612091	65	2	124	30	3.21	0.509	-0.758	2.318	2.098
23	CA1	62977	612091	65	2	124	10	3.12	0.719	-0.858	2.743	6.690
24	CA1	62977	612103	30	3	120	30	3.35	0.286	-0.551	2.891	0.616
25	CA1	62977	612103	30	3	120	10	3.12	0.629	-0.714	1.920	0.713
26	CA1	62977	612122	45	2	124	30	3.17	0.396	-0.203	-0.054	1.243
27	CA1	62977	612122	45	2	124	10	3.16	0.403	-0.999	6.344	4.378
28	CA1	62977	612123	35	2	122	30	2.43	1.037	0.078	-1.541	0.714
29	CA1	62977	612123	35	2	122	10	2.91	0.922	-0.779	1.925	8.600
30	CA1	62977	612142	50	2	122	30	3.18	0.840	-1.079	5.049	1.204
31	CA1	62977	612142	50	2	122	10	3.08	0.699	-0.815	3.787	4.627
32	CA1	62977	612143	35	2	118	10	2.78	0.632	-0.302	0.454	6.829
33	CA1	63077	613043	45	2	114	30	3.04	0.649	-0.832	1.105	0.726
34	CA1	63077	613043	45	2	114	10	3.14	0.510	-0.367	0.235	1.937
35	CA1	63077	613053	50	3	116	30	3.07	0.562	-0.522	0.710	0.311
36	CA1	63077	613053	50	3	116	10	3.21	0.679	-0.725	1.546	0.633
37	CA1	63077	613063	40	3	117	30	3.52	0.170	-0.062	-0.856	0.112
38	CA1	63077	613063	40	3	117	10	3.39	0.238	-0.017	-0.931	0.166
39	CA1	63077	613072	35	2	118	30	3.24	0.461	-0.572	0.862	0.111
40	CA1	63077	613072	35	2	118	10	3.09	0.636	-0.757	1.680	0.099
41	CA1	63077	613073	40	2	122	30	3.12	0.754	-0.787	2.025	1.083
42	CA1	63077	613073	40	2	122	10	3.30	0.476	-0.153	-0.640	2.411
43	CA1	63077	613082	30	2	121	30	3.27	0.541	-0.427	0.537	1.603
44	CA1	63077	613082	30	2	121	10	3.23	0.432	-0.794	7.870	0.806
45	CA1	63077	613084	40	2	125	30	3.14	0.442	-0.214	0.014	2.255
46	CA1	63077	613084	40	2	125	10	3.05	0.608	-0.744	3.673	3.292
47	CA1	63077	613102	40	2	119	10	3.34	0.317	-0.075	-0.756	.
48	CA1	63077	613101	40	2	122	10	2.52	0.646	-0.223	-0.412	.
49	CA1	63077	613033	40	2	110	10	2.46	0.603	-0.337	1.001	0.797
50	CA1	70277	615042	40	2	121	30	3.04	0.707	-0.750	2.424	1.680
51	CA1	70277	615042	40	2	121	10	3.16	0.417	-0.390	0.930	2.218
52	CA1	70277	615043	65	2	124	30	3.25	0.864	-0.964	2.912	0.738
53	CA1	70277	615043	65	2	124	10	3.14	0.687	-0.600	1.677	3.036
54	CA1	70277	615123	50	2	125	30	3.08	0.783	-1.126	5.162	1.536

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ORS	STA	DATE	SERIES	WVMT	WVTYPE	DIS	HT	MZ	STDEV	SKEW	KURT	SS
55	CA1	70277	615123	50	2	125	10	3.09	0.335	-0.128	0.144	5.342
56	CA1	70477	619013	75	1	74	30	3.06	0.512	-0.874	2.742	0.132
57	CA1	70477	619014	75	1	74	10	3.05	0.524	-0.501	0.750	0.182
58	CA1	70477	619102	22	1	25	10	3.12	0.235	-0.167	-0.523	.
59	CA1	70477	621012	105	2	120	60	3.17	0.560	-1.427	11.791	.
60	CA1	70977	621012	105	2	120	30	3.14	0.594	-1.017	4.195	.
61	CA1	70977	621012	105	2	120	10	3.26	0.355	-0.466	3.930	.
62	CA1	70977	621013	100	2	115	60	3.44	0.310	-0.173	-0.589	.
63	CA1	70977	621013	100	2	115	30	3.19	0.393	-0.206	-0.331	.
64	CA1	70977	621013	100	2	115	10	3.13	0.453	-1.000	7.644	.
65	P11	61877	607043	8	2	65	10	3.16	0.398	-0.164	0.279	.
66	P11	61877	607042	35	1	93	10	3.09	0.689	-0.992	4.600	0.332
67	P11	61877	607122	35	1	120	10	3.31	0.520	-0.924	4.586	1.154
68	P11	61877	607123	35	1	115	10	3.47	0.515	-1.937	20.072	0.556
69	P11	62177	610023	35	2	52	10	2.85	0.559	-0.444	0.105	1.948
70	P11	62177	610033	35	2	51	10	2.88	0.547	-0.534	1.708	6.717
71	P11	62177	610043	25	2	55	10	2.68	0.582	-0.615	2.990	2.696
72	P11	62177	610053	15	3	54	10	3.08	0.472	-0.328	0.830	1.595
73	HU2	61277	602022	30	2	102	10	2.99	0.541	-0.030	-0.692	0.441
74	HU2	61277	602042	25	3	101	30	3.08	0.645	-0.249	-0.804	0.099
75	HU2	61277	602042	25	3	101	10	3.04	0.629	-0.269	-0.756	0.115
76	HU2	61277	602052	50	2	104	30	3.44	0.211	-0.038	-1.594	0.275
77	HU2	61277	602052	50	2	104	10	3.39	0.398	-0.380	0.336	0.528
78	HU2	61277	602062	55	3	100	30	3.43	0.378	-0.296	-1.064	0.120
79	HU2	61277	602062	55	3	100	10	3.63	0.237	-0.059	-0.903	0.220
80	HU2	61277	602102	30	2	75	10	3.23	0.381	0.033	-1.049	0.711
81	HU2	61577	605022	70	2	94	30	3.66	0.124	-0.194	-1.336	1.350
82	HU2	61577	605022	70	2	94	10	3.41	0.518	-1.197	6.313	7.410
83	HU2	61577	605023	70	2	94	30	3.63	0.326	-2.466	47.523	1.209
84	HU2	61577	605023	70	2	94	10	3.34	0.630	-1.170	7.399	1.463
85	HU2	61577	605021	70	2	94	30	3.29	0.741	-1.386	8.045	0.801
86	HU2	61577	605021	70	2	94	10	3.37	0.585	-1.262	6.793	1.266
87	HU2	61577	605032	70	2	94	10	3.14	0.864	-0.989	4.083	1.766
88	HU2	61577	605032	70	2	94	30	3.17	0.893	-0.813	1.086	1.049
89	HU2	61577	605033	70	2	94	10	3.30	0.507	-0.973	5.404	1.453
90	HU2	61577	605031	70	2	94	30	3.38	0.554	-0.770	2.685	1.810
91	HU2	61577	605031	70	2	94	10	3.28	0.501	-0.871	4.967	2.694
92	HU2	61577	605042	45	2	100	30	3.32	0.559	-0.878	4.308	3.121
93	HU2	61577	605042	45	2	100	10	3.45	0.367	-1.037	7.446	9.215
94	HU2	61577	605043	45	2	100	30	3.22	0.639	-0.727	1.873	2.206
95	HU2	61577	605043	45	2	100	10	3.15	0.801	-0.893	3.040	5.778
96	HU2	61577	605041	45	2	100	30	3.45	0.277	0.019	0.063	9.221
97	HU2	61577	605041	45	2	100	10	3.20	0.747	-0.996	4.255	9.999
98	HU2	61577	605052	40	2	101	30	3.64	0.165	0.356	-0.563	0.541
99	HU2	61577	605052	40	2	101	10	3.36	0.692	-1.501	10.237	0.599
100	HU2	61577	605053	40	2	101	30	3.27	0.646	-0.411	-0.157	0.650
101	HU2	61577	605053	40	2	101	10	3.01	1.158	-0.704	0.620	1.090
102	HU2	61577	605062	35	2	101	30	3.52	0.211	0.235	-0.343	1.945
103	HU2	61577	605062	35	2	101	10	3.34	0.433	-1.432	13.635	2.614
104	HU2	61577	605061	35	2	101	30	3.38	0.362	-0.445	2.934	0.895
105	HU2	61577	605061	35	2	101	10	3.47	0.307	-0.354	1.355	1.753
106	HU2	61577	605073	30	2	101	10	3.06	0.901	-0.671	0.945	2.642
107	HU2	61577	605071	30	2	101	30	3.24	0.464	-1.426	18.237	2.158
108	HU2	61577	605073	30	2	101	30	3.34	0.310	-0.334	0.634	1.513

ORS	STA	DATE	SERIES	WVMT	WVTYPE	DIS	HT	MZ	STDEV	SKEW	KURT	SS
109	HU2	61577	605071	30	2	101	10	3.15	0.782	-1.065	4.675	2.618
110	HU2	62877	611023	50	2	140	10	3.24	0.598	-0.806	4.198	1.342
111	HU2	62877	611052	20	2	111	10	3.62	0.263	0.005	-1.167	1.558
112	HU2	62877	611053	20	2	111	10	2.74	0.819	-0.533	0.933	1.094
113	HU2	62877	611062	15	3	115	10	2.31	0.970	0.047	-1.272	7.071
114	HU2	62877	611063	30	3	109	10	3.25	0.475	-0.426	2.307	4.197
115	HU2	62877	611061	20	3	112	30	3.53	0.384	-0.349	-0.255	0.575
116	HU2	62877	611061	20	3	112	10	3.35	0.543	-1.287	10.327	0.981
117	HU2	62877	611073	15	3	106	10	1.14	0.866	0.726	1.590	0.429
118	HU2	62877	611082	20	5	108	10	3.09	0.672	-0.308	-0.766	6.154
119	HU2	62877	611083	20	5	109	10	3.27	0.565	-0.992	5.955	2.592
120	HU2	62877	611081	25	5	112	30	2.85	0.892	-0.530	0.405	0.543
121	HU2	62877	611081	25	5	112	10	3.21	0.797	-1.151	5.535	1.469
122	HU2	70177	614042	65	3	140	30	3.08	0.899	-0.569	0.596	0.580
123	HU2	70177	614042	65	3	140	10	3.26	0.678	-0.776	1.806	0.604
124	HU2	70177	614043	60	3	145	30	3.26	0.889	-1.317	5.414	0.555
125	HU2	70177	614043	60	3	145	10	3.44	0.391	-0.676	3.771	0.688
126	HU2	70177	614053	35	1	150	30	3.25	0.652	-0.886	2.785	0.741
127	HU2	70177	614053	35	1	150	10	3.50	0.328	-0.411	-0.251	2.183
128	HU2	70177	614062	40	3	151	30	2.89	0.814	-0.301	-1.267	0.799
129	HU2	70177	614062	40	3	151	10	3.31	0.534	-0.842	2.986	0.982
130	HU2	70177	614064	70	3	154	30	3.31	0.617	-0.756	2.113	0.837
131	HU2	70177	614063	70	3	155	10	3.26	0.674	-0.864	3.175	1.248
132	HU2	70177	614082	25	3	140	10	3.27	0.690	-0.621	0.453	1.073
133	HU2	70177	614083	53	3	150	30	3.34	0.442	-0.736	2.220	0.856
134	HU2	70177	614083	53	3	150	10	3.39	0.247	0.216	-0.412	2.575
135	HU2	70177	614081	80	3	160	60	3.47	0.257	-0.285	-0.434	0.816
136	HU2	70177	614081	80	3	160	30	3.35	0.554	-1.632	13.513	1.034
137	HU2	70177	614081	80	3	160	10	3.34	0.643	-1.614	11.344	1.587
138	HU2	70177	614092	40	1	150	30	3.27	0.397	-0.238	1.175	1.706
139	HU2	70177	614092	40	1	150	10	3.21	0.770	-0.905	3.265	2.435

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SUSPENDED SEDIMENT SIZE DISTRIBUTION

144

WAVE PARAMETERS

Breaker Type spilling
 Breaker Height (H_b) 25 cm
 Depth at Breaking (d_b) 30 cm
 d_b/H_b 1.2

SUSPENDED SEDIMENT CONCENTRATION

conc. (g/l)

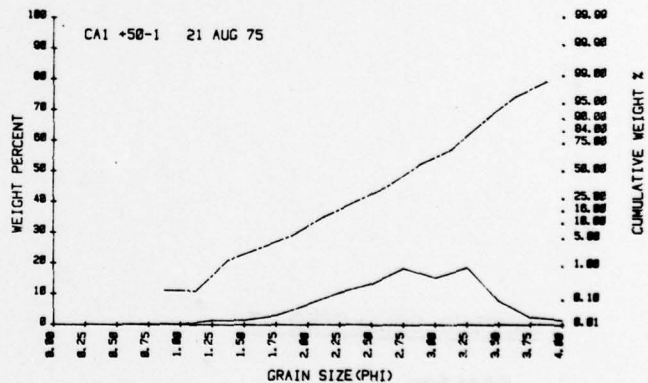
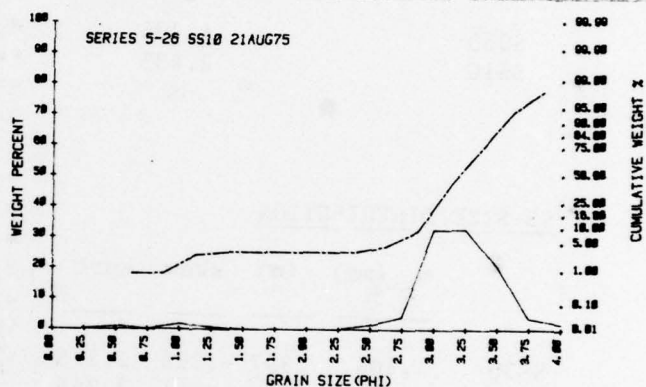
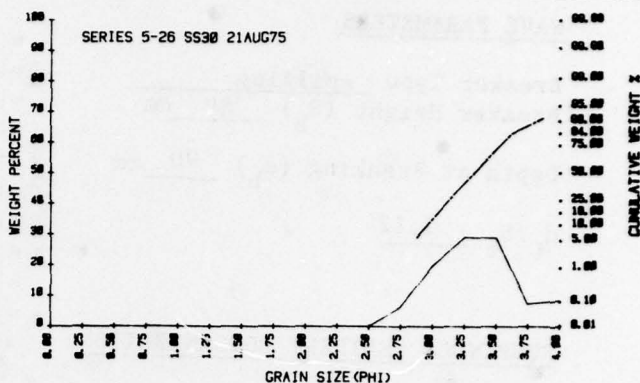
SS30 .583
 SS10 1.513

SS SIZE DISTRIBUTION

	m_z (mm)	(σ)	skew	kurt
		(phi)		
SS30	.099	.323	.137	
SS10	.113	.493	-1.507	
BED	.143	.710	-0.328	.514

SAMPLES COLLECTED AT

Station CA-1
 Date 21 Aug. 75
 Time 1045



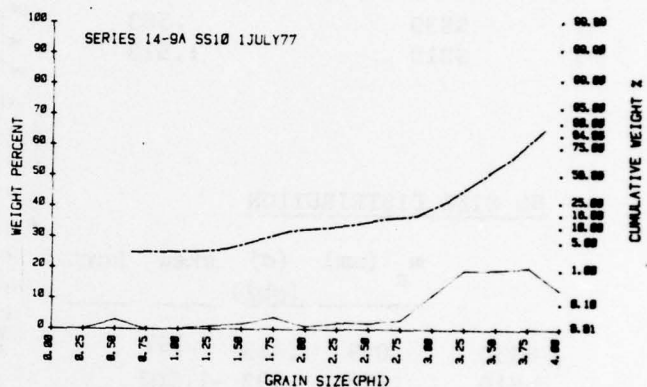
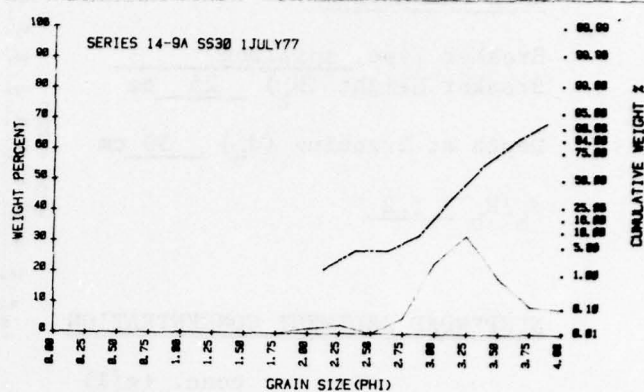
WAVE PARAMETERSBreaker Type spillingBreaker Height (H_b) 80 cmDepth at Breaking (d_b) 90 cm d_b/H_b 1.12SUSPENDED SEDIMENT CONCENTRATION

conc. (g/l)

SS30	1.706
SS10	2.435

SS SIZE DISTRIBUTION

	m_z (mm)	(σ)	skew	kurt
SS30	.104	.397	-.238	1.175
SS10	.107	.770	-.905	3.265

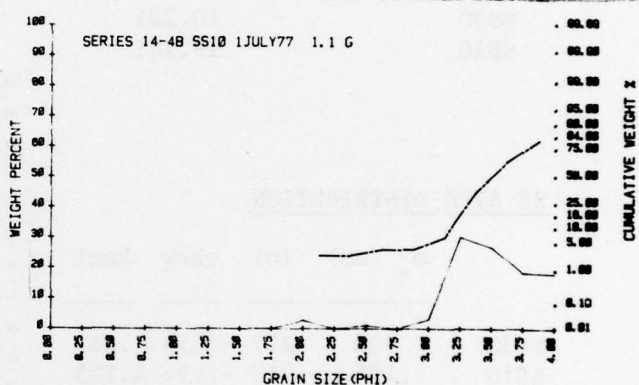
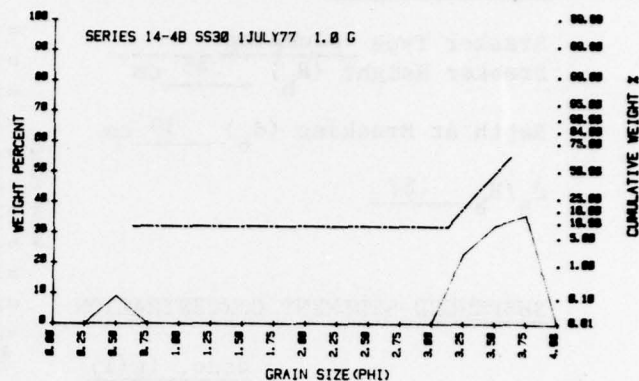
SAMPLES COLLECTED ATStation BU-2Date 1 July 77Time 1422

WAVE PARAMETERSBreaker Type transitionBreaker Height (H_b) 90 cmDepth at Breaking (d_b) 90 cm d_b/H_b 1.00SUSPENDED SEDIMENT CONCENTRATIONconc. (g/l)

SS30	.555
SS10	.688

SS SIZE DISTRIBUTION

	m_z (mm)	(σ)	skew	kurt
SS30	.104	.889	-1.317	.596
SS10	.089	.391	-0.676	1.806

SAMPLES COLLECTED ATStation BU-2Date 1 July 1977Time 1220

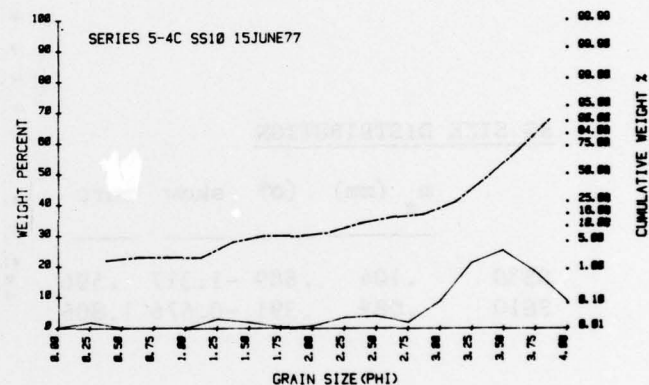
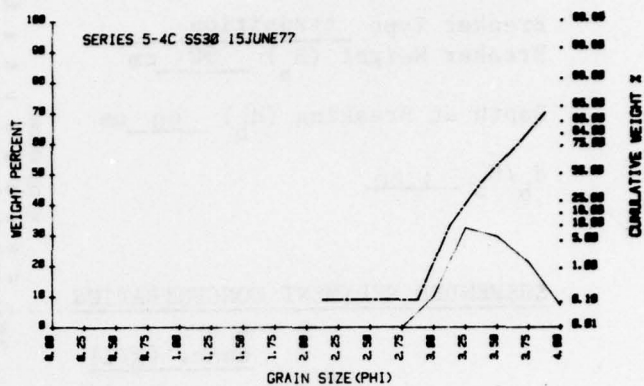
WAVE PARAMETERSBreaker Type plungingBreaker Height (H_b) 45 cmDepth at Breaking (d_b) 30 cm d_b/H_b .67SUSPENDED SEDIMENT CONCENTRATION

conc. (g/l)

SS30	10.221
SS10	19.161

SS SIZE DISTRIBUTION

	m_z (mm)	(σ)	skew	kurt
SS30	.108	.277	.019	.063
SS10	.105	.747	-.996	4.225

SAMPLES COLLECTED ATStation BU-2Date 15 June 1977Time 1150

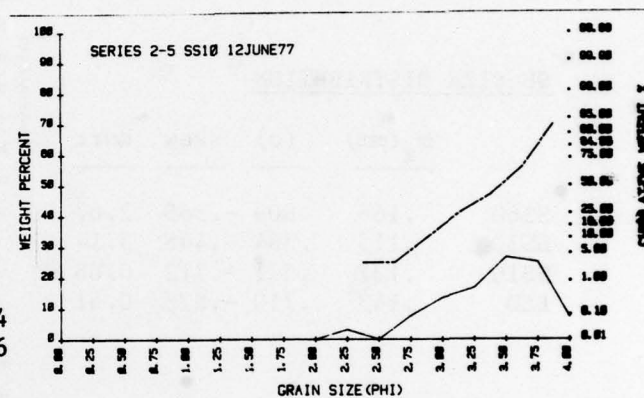
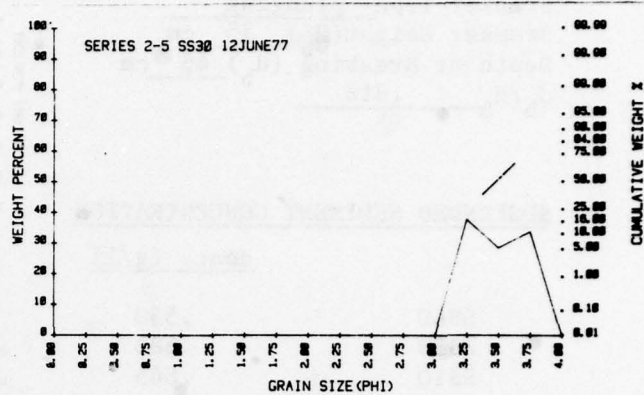
WAVE PARAMETERSBreaker Type non-breakingBreaker Height (H_b) 50 cmDepth at Breaking (d_b) 40 cm d_b/H_b .80SUSPENDED SEDIMENT CONCENTRATIONconc. (g/l)

SS30 .275

SS10 .528

SS SIZE DISTRIBUTION

	m_z (mm)	(σ)	skew	kurt
SS30	.089	.211	.038	-1.594
SS10	.095	.398	-.380	.336

SAMPLES COLLECTED ATStation BU-2Date 12 June 1977Time 1240

WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 55 cm
 Depth at Breaking (d_b) 45 cm
 d_b/H_b .818

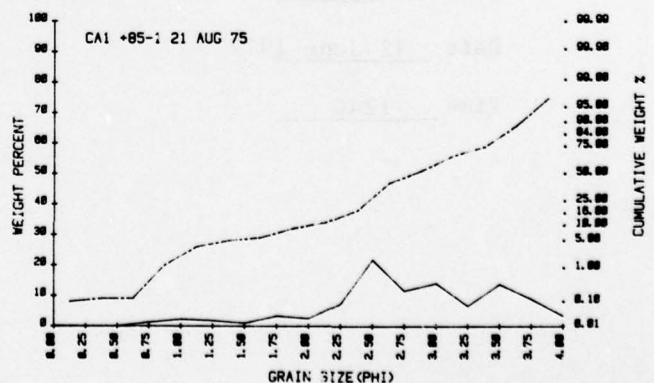
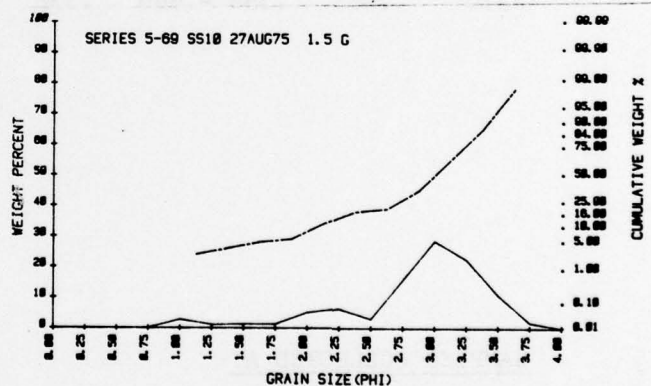
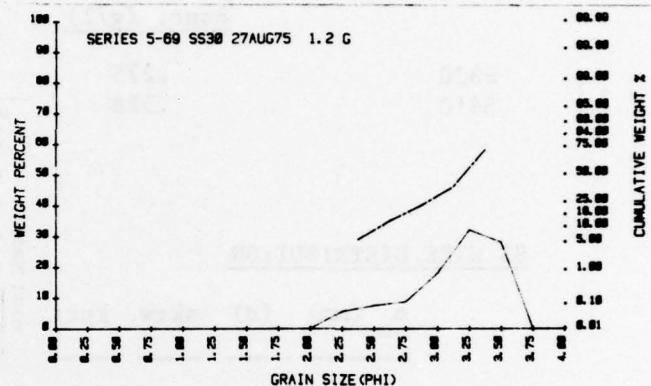
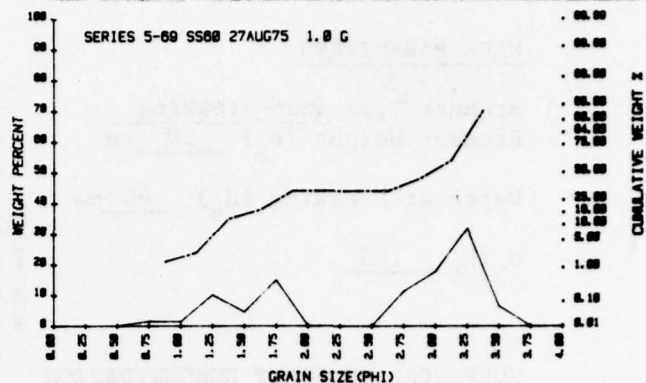
SUSPENDED SEDIMENT CONCENTRATION

conc. (g/l)

SS60 .530
 SS30 .683
 SS10 .845

SS SIZE DISTRIBUTION

	m_z (mm)	(o)	skew	kurt
SS60	.166	.809	-.365	2.67
SS30	.115	.364	-.448	3.14
SS10	.137	.571	-.712	0.88
BED	.143	.710	-.328	0.51

SAMPLES COLLECTED ATStation CA1Date 27 Aug 75Time 1045

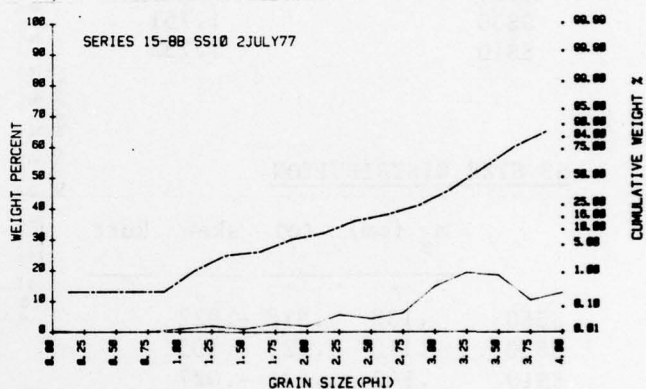
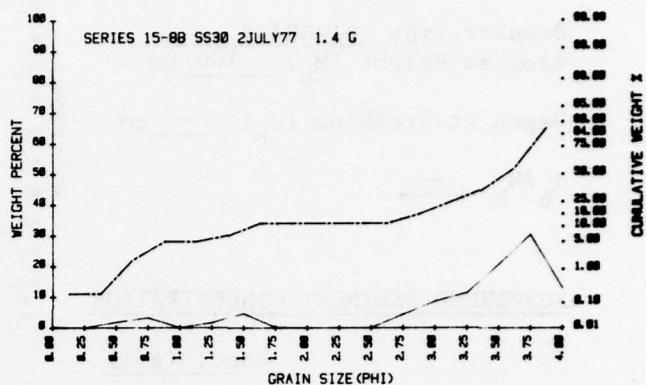
WAVE PARAMETERSBreaker Type plungingBreaker Height (H_b) 65 cmDepth at Breaking (d_b) 40 cm d_b/H_b .62SUSPENDED SEDIMENT CONCENTRATION

conc. (g/l)

SS30	.738
SS10	3.036

SS SIZE DISTRIBUTION

	m_z (mm)	(σ)	skew	kurt
SS30	.105	.864	-.969	2.428
SS10	.113	.687	-.600	.930

SAMPLES COLLECTED ATStation CA-1Date 2 July 1977Time 1409

WAVE PARAMETERS

Breaker Type plunging
 Breaker Height (H_b) 100 cm
 Depth at Breaking (d_b) - cm
 d_b/H_b ---

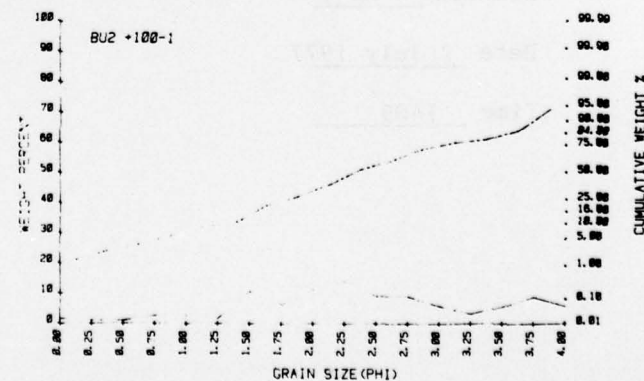
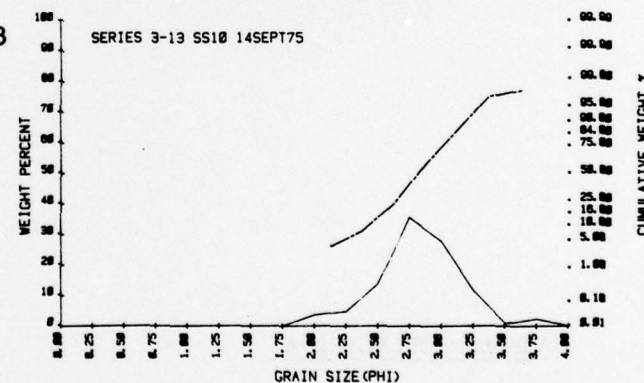
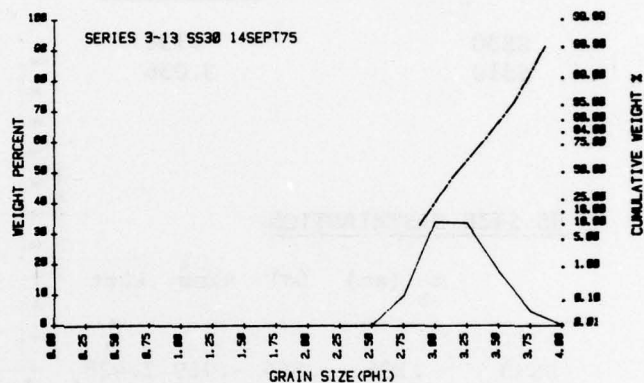
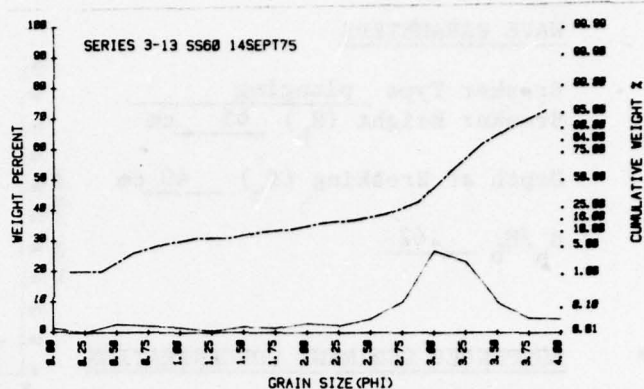
SUSPENDED SEDIMENT CONCENTRATION

conc. (g/l)

SS60 1.539
 SS30 1.751
 SS10 7.732

SS SIZE DISTRIBUTION

	m_z (mm)	(σ)	skew	kurt
SS60	.138	.818	-.822	
SS30	.110	.225	.135	
SS10	.142	.331	-.027	
BED	.190	.940	-.076	-.363

SAMPLES COLLECTED ATStation BU-2Date 14 Sept. 75Time 0955

WAVE PARAMETERS

Breaker Type _____

Breaker Height (H_b) 135 cmDepth at Breaking (d_b) 130 cm d_b/H_b .96SUSPENDED SEDIMENT CONCENTRATION

conc. (g/l)

SS60 1.285

SS30 1.592

SS10 11.334

SS SIZE DISTRIBUTION

	m_z (mm)	(σ)	skew	kurt
SS60	.096	.183	.112	
SS30	.101	.392	-.645	
SS10	.174	.586	-.327	
BED	.135	.745	-.781	2.58

SAMPLES COLLECTED ATStation CA-1Date 17 Oct. 75Time 1500